GENERAL NOTES

1. STRUCTURAL DESIGN IS IN ACCORDANCE WITH THE PROVISIONS OF THE 2018 INTERNATIONAL BUILDING CODE.

2. THE BUILDING STRUCTURE HAS BEEN DESIGNED TO RESIST THE FOLLOWING CODE PRESCRIBED LOADS:

   LIVE LOADS

ROOF	20 PSF
FLOOR	100 PSF

   SNOW LOADS

GROUND SNOW LOAD, Pg.	5 PSF
SNOW IMPORTANCE FACTOR,Is	1.1
SNOW EXPOSURE FACTOR, Ce	0.9
THERMAL FACTOR, Ct	1.0

   WIND LOADS (BUILDINGS)

ULTIMATE DESIGN WIND SPEED (RISK CATEGORY III)	112 MPH
EXPOSURE CATEGORY	B
SURFACE ROUGHNESS	B

   SEISMIC LOADS

OCCUPANCY CATEGORY	III
SEISMIC IMPORTANCE FACTOR, IE	1.25
SPECTRAL RESPONSE COEFFICIENT, SS	7.7%g
SPECTRAL RESPONSE COEFFICIENT, S1	4.4%g
SITE CLASS	C
SEISMIC DESIGN CATEGORY	A

3. THE STRUCTURAL DRAWINGS AND SPECIFICATIONS REPRESENT THE FINISHED STRUCTURE. THEY DO NOT INDICATE THE METHODS OF CONSTRUCTION UNLESS SO STATED OR NOTED. THE CONTRACTOR SHALL PROVIDE ALL MEASURES NECESSARY TO PROTECT THE WORKMEN AND OTHER PERSONS DURING CONSTRUCTION.

4. THE STRUCTURAL DRAWINGS SHALL NOT BE SCALED FOR DETERMINATION OF QUANTITY, LENGTH OR FIT OF MATERIALS.

5. PRINCIPAL OPENINGS ARE INDICATED ON THE STRUCTURAL DRAWINGS. REFER TO ARCHITECTURAL, MECHANICAL AND ELECTRICAL DRAWINGS FOR SLEEVES, BLOCKOUTS, INSERTS, CURBS, OPENINGS AND SLAB DEPRESSIONS NOT SHOWN.

6. CONTRACTOR SHALL COMPARE STRUCTURAL AND ARCHITECTURAL DRAWINGS AND REPORT ANY DISCREPANCY TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS.

7. CONTRACTOR SHALL INSURE THAT CONSTRUCTION MATERIALS WHOSE WEIGHT EXCEEDS THE DESIGN LIVE LOADS INDICATED ON THE STRUCTURAL DRAWINGS ARE NOT STORED ON STRUCTURALLY SUPPORTED FLOOR OR ROOF FRAMING.

8. THE CONTRACTOR SHALL PROVIDE TEMPORARY ERECTION BRACING AND SHORING OF ALL STRUCTURAL WORK AS REQUIRED FOR STABILITY OF THE STRUCTURE DURING ALL PHASES OF CONSTRUCTION. THE CONTRACTOR SHALL IMMEDIATELY NOTIFY THE ENGINEER OF ANY CONDITION WHICH, IN HIS OR HER OPINION, MIGHT ENDANGER THE STABILITY OF THE STRUCTURE OR CAUSE DISTRESS IN THE STRUCTURE.

9. LOADINGS FOR MECHANICAL EQUIPMENT ARE BASED ON THE UNIT(S) SHOWN ON THE STRUCTURAL DRAWINGS. ANY CHANGES IN TYPE, SIZE, WEIGHT OR NUMBER OF UNIT(S) SHALL BE REPORTED TO THE ARCHITECT PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS OR MECHANICAL EQUIPMENT.

10. REPRODUCTION OF THE STRUCTURAL DRAWINGS, EITHER IN PART OR IN WHOLE, FOR SUBMITTALS OR SHOP DRAWINGS SIGNIFIES ACCEPTANCE OF INFORMATION SHOWN AS CORRECT AND OBLIGES THE USER TO ANY EXPENSE, REAL OR IMPLIED, ARISING FROM THEIR USE.

11. CONTRACTOR SHALL SCHEDULE SITE OBSERVATION VISITS WITH THE ENGINEER OF RECORD AND/OR TESTING LABORATORY A MINIMUM OF FORTY-EIGHT HOURS PRIOR TO THE REQUIRED TIME OF THE VISIT.

12. CONTRACTOR SHALL ALLOW TEN (10) WORKING DAYS FOR THE ENGINEER TO REVIEW EACH STRUCTURAL SUBMITTAL OR SHOP DRAWING.

13. FIELD VERIFY ALL HORIZONTAL AND VERTICAL DIMENSIONS OF EXISTING STRUCTURE PRIOR TO FABRICATION OR INSTALLATION OF STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

FOUNDATION NOTES

1. THE FOUNDATION DESIGN IS BASED ON THE PROJECT GEOTECHNICAL REPORT PREPARED BY D&S ENGINEERING LABS(D&S PROJECT NO. G21-2350) DATED MARCH 18, 2022.

2. THE FOUNDATION DESIGN IS BASED ON A POTENTIAL VERTICAL MOVEMENT, PVM, ON THE ORDER OF ONE (1) INCH OR LESS. IF THIS VALUE IS NOT ACCEPTABLE TO THE OWNER OR TENANTS, THE FOUNDATION DESIGN MUST BE REVISED.

2. THE FOUNDATION SHALL CONSIST OF AUGER-EXCAVATED, STRAIGHT SHAFT REINFORCED CONCRETE PIERS. REFER TO TYPICAL PIER DETAIL FOR BEARING STRATA. PIERS HAVE BEEN PROPORTIONED FOR THE FOLLOWING:

END BEARING	10,000 PSF
SKIN FRICTION (COMPRESSION)	2,000 PSF

3. ALL UNEXPOSED SURFACES OF FOOTINGS/GRADE BEAMS SHALL BE EARTH-FORMED. PROVIDE FORMWORK FOR ALL EXPOSED SURFACES AND THE UPPER (12) INCHES OF ALL EXTERIOR FOOTINGS/GRADE BEAMS.

4. CORRUGATED PAPER FORMS, AS MANUFACTURED BY SUREVOID PRODUCTS INC., SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER TO PROVIDE A NOMINAL SIX (6) INCH VOID BENEATH ALL GRADE BEAMS. SURE RETAINER BY MOTZBLOCK PLASTIC BACKFILL RETAINER BOARDS, AS MANUFACTURED BY SUREVOID PRODUCTS, INC., SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER CONTINUOUSLY ALONG EACH SIDE OF ALL GRADE BEAMS.

5. THE BUILDING SLAB ON GRADE SHALL BE PLACED ON A VAPOR BARRIER/ RETARDER OVER TWELVE (12) INCHES OF NON-EXPANSIVE OF SELECT FILL OVER A MINIMUM OF TWELVE (12) INCHES OF REWORKED ON-SITE SOILS IN ACCORDANCE WITH THE PROJECT GEOTECHNICAL REPORT.

6. VAPOR BARRIER/RETARDER SHALL BE IN COMPLIANCE WITH ASTM E 1745 CLASS A, HAVE A MINIMUM THICKNESS OF FIFTEEN (15) MILS AND A PERMEANCE AS TESTED AFTER MANDATORY CONDITIONING (ASTM E 154 SECTIONS 8, 11, 12, 13) LESS THAN 0.01 PERMS (GRAINS/(FT2*HR*IN.HG)) PER ASTM E 96 OR F 1249. MEMBRANE, TAPE, AND ACCESSORIES SHALL BE INSTALLED IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.

7. INFORMATION ABOVE IS PRESENTED ONLY AS A SUMMARY OF THE PROJECT GEOTECHNICAL REPORT. THE CONTRACTOR IS RESPONSIBLE FOR REVIEWING AND COMPLYING WITH THE RECOMMENDATIONS CONTAINED IN THE PROJECT GEOTECHNICAL REPORT. THE STRUCTURAL ENGINEER IS NOT RESPONSIBLE FOR SUBSURFACE CONDITIONS ENCOUNTERED IN THE FIELD DIFFERENT TO THOSE ASSUMED FOR DESIGN.

8. IF MORE THAN SIX (6) MONTHS ELAPSE FROM THE ISSUE DATE OF THE CONSTRUCTION DOCUMENTS TO THE COMMENCEMENT OF CONSTRUCTION, IT IS RECOMMENDED THAT THE BUILDING OWNER CONSULT WITH THE PROJECT GEOTECHNICAL ENGINEER TO DETERMINE IF THE FOUNDATION DESIGN RECOMMENDATIONS ARE CONSISTENT WITH THE CURRENT SOIL CONDITIONS.

STRUCTURAL CONCRETE NOTES

1. ALL CONCRETE WORK SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF ACI 301 AND ACI 318. ALL CONCRETE SHALL BE LABORATORY DESIGNED AND CONTROLLED.

2. UNLESS NOTED OTHERWISE, ALL CONCRETE SHALL HAVE SAND AND GRAVEL OR CRUSHED STONE COARSE AGGREGATES AND A CORRESPONDING TWENTY-EIGHT (28) DAY COMPRESSIVE STRENGTH OF 3,000 PSI. ALL CONCRETE THAT WILL BE PERMANENTLY EXPOSED TO WEATHER SHALL CONTAIN AN AIR ENTRAINING AGENT THAT PROVIDES FOUR (4) TO SIX (6) PERCENT AIR BY VOLUME.

3. VAULT CONCRETE SHALL HAVE SAND AND PEA-GRAVEL COARSE AGGREGATES AND A CORRESPONDING TWENTY-EIGHT (28) DAY COMPRESSIVE STRENGTH OF 4,000 PSI.

4. CONCRETE PROTECTION FOR STEEL REINFORCEMENT SHALL BE AS FOLLOWS (SEE ACI 318, SECTION 7.7 FOR CONDITIONS NOT INDICATED):

ALL CONCRETE PLACED AGAINST SOIL	3"
SLABS ON GRADE	AT SLAB MID-DEPTH
WALLS	1 1/2" SIDES
FOOTINGS/GRADE BEAMS	3" BOTTOM AND SIDES, 1 1/2" TOP

5. LOCATE JOINTS TO LEAST IMPAIR STRENGTH AND APPEARANCE OF STRUCTURE. LOCATE HORIZONTAL JOINTS IN CONCRETE ONLY WHERE THEY NORMALLY OCCUR OR WHERE INDICATED ON PLAN. LOCATE VERTICAL JOINTS IN THE MIDDLE THIRD OF SPAN.

6. ROUGHEN SURFACE OF HORIZONTAL OR NEARLY HORIZONTAL CONSTRUCTION JOINTS SO THAT AGGREGATE SHALL BE EXPOSED UNIFORMLY, LEAVING NO LAITANCE, LOOSENED PARTICLES OR DAMAGED CONCRETE.

7. THE PLACEMENT OF SLEEVES OR OPENINGS THRU CONCRETE MEMBERS IS PROHIBITED UNLESS SPECIFICALLY INDICATED ON THE STRUCTURAL DRAWINGS OR APPROVED IN WRITING BY THE ENGINEER OF RECORD.

8. PROVIDE CHAMFERS AND REVEALS AS INDICATED IN THE ARCHITECTURAL DRAWINGS.

9. THE BUILDING OWNER SHALL SECURE AN INDEPENDENT TESTING LABORATORY TO PERFORM AT LEAST ONE COMPRESSIVE STRENGTH TEST FOR EACH ONE HUNDRED (100) CUBIC YARDS, OR FRACTION THEREOF, OF EACH MIX DESIGN OF CONCRETE PLACED ON ANY ONE DAY. THE LABORATORY SHALL RECORD THE MIX DESIGN, LOCATION OF PLACEMENT, AND SLUMP OF EACH SPECIMEN.

10. A COMPRESSIVE STRENGTH TEST SHALL BE COMPRISED OF FOUR (4) 6"X12" OR FIVE (5) 4"X8" CYLINDER SPECIMENS OBTAINED IN ACCORDANCE WITH ASTM C31. ONE (1) CYLINDER SPECIMEN SHALL BE TESTED AT SEVEN (7) DAYS FOR INFORMATION AND TWO (2) 6"X12" CYLINDER SPECIMENS OR THREE (3) 4"X8" CYLINDER SPECIMENS SHALL BE TESTED AT TWENTY-EIGHT (28) DAYS FOR ACCEPTANCE. THE REMAINING CYLINDER SPECIMEN SHALL BE HELD FOR TESTING AS DIRECTED.

REINFORCING STEEL NOTES

1. ALL DETAILING OF STEEL REINFORCEMENT AND ACCESSORIES SHALL CONFORM TO ACI COMMITTEE 315 PUBLICATION SP-66, "ACI DETAILING MANUAL."

2. DEFORMED BAR REINFORCEMENT SHALL BE DOMESTIC NEW BILLET STEEL IN CONFORMANCE WITH ASTM A615, GRADE 60.

3. WELDED WIRE FABRIC SHALL BE ELECTRICALLY WELDED, COLD-DRAWN WIRE IN CONFORMANCE WITH ASTM A185, GRADE 65. WELDED WIRE FABRIC SHALL BE PLACED IN FLAT SHEETS ONLY.

4. LAP WELDED WIRE FABRIC AT LEAST 1 1/2 SQUARES PLUS WIRE END EXTENSIONS BUT NOT LESS THAN TWELVE (12) INCHES, UNLESS NOTED OTHERWISE. EXTEND MESH ACROSS SUPPORTING BEAMS AND WALLS.

ADHESIVE ANCHOR AND DOWEL NOTES

1. WHERE NOTED IN THE PLANS AND DETAILS, ADHESIVE ANCHORS AND DOWELS SHALL BE INSTALLED WITH HILTI HY200 SAFE SET EPOXY IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER.

2. ADHESIVE ANCHORS AND/OR DOWELS NOT NOTED IN THE PLANS AND DETAILS ARE NOT ALLOWED WITHOUT PRIOR WRITTEN CONSENT OF THE STRUCTURAL ENGINEER OF RECORD.

3. UNLESS NOTED OTHERWISE, THE MINIMUM EMBEDMENT DEPTH OF ADHESIVE ANCHORS AND DOWELS SHALL BE AS FOLLOWS:

ANCHOR/DOWEL	EMBEDMENT
3/8" DIA. OR #3 BAR	4 1/2"
1/2" DIA. OR #4 BAR,	6"
5/8" DIA. OR #5 BAR	9 5/8"
3/4" DIA. OR #6 BAR	11 1/4"
7/8" DIA. OR #7 BAR	13 1/8"
1" DIA. OR #8 BAR	15"

STRUCTURAL STEEL NOTES

1. ALL STRUCTURAL STEEL DETAILING, FABRICATION AND INSTALLATION SHALL CONFORM TO THE STANDARDS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).

2. PROVIDE NEW DOMESTIC STRUCTURAL STEEL IN ACCORDANCE WITH THE FOLLOWING:

WIDE FLANGE SHAPES	ASTM A992
CHANNELS, PLATES AND ANGLES	ASTM A36
STEEL TUBE	ASTM A500, GRADE B
STEEL PIPE	ASTM A53 (TYPES E OR S), GRADE B

3. THE DETAILER SHALL DESIGN ALL CONNECTIONS TO RESIST FIFTY (50) PERCENT OF THE ALLOWABLE SHEAR CAPACITY OF THE BEAM, UNLESS NOTED OTHERWISE. AS A MINIMUM, PROVIDE THE NUMBER OF BOLTS SHOWN BELOW FOR EACH BEAM SIZE:

BEAM SIZE	NUMBER OF BOLTS
W8 & W10	2 MINIMUM
W12, W14, W16	3 MINIMUM
W18 & W21	4 MINIMUM
W24 & W27	5 MINIMUM

4. CONNECTION BOLTS FOR STRUCTURAL STEEL MEMBERS SHALL BE 3/4 INCH DIAMETER ASTM A325-N BOLTS, UNLESS NOTED OTHERWISE.

5. ANCHOR BOLTS SHALL BE UNFINISHED THREADED FASTENERS THAT CONFORM TO ASTM F1554, GRADE 36 BOLTS AND NUTS WITH HEXAGONAL HEADS.

6. SPLICING OF STRUCTURAL STEEL MEMBERS IS PROHIBITED EXCEPT AS SPECIFICALLY INDICATED IN STRUCTURAL DRAWINGS.

7. ERECT ALL STEEL BEAMS WITH NATURAL OR SPECIFIED CAMBER UP.

8. UNLESS NOTED OTHERWISE, HOT DIP GALVANIZE ALL STRUCTURAL STEEL MEMBERS AND EMBEDS EXPOSED TO WEATHER OR SOIL AND WHERE INDICATED ON DRAWINGS. GALVANIZING SHALL CONFORM TO ASTM A123.

9. TOUCH UP FIELD WELDS ON GALVANIZED ITEMS WITH PAINT CONFORMING TO TT-P-641.

WELDING NOTES

1. WELDING OF STRUCTURAL STEEL SHALL CONFORM TO AWS D1.1. USE E70XX ELECTRODES FOR FIELD AND SHOP WELDS. USE ONLY LOW-HYDROGEN ELECTRODES ON ASTM A242, A514, A572 AND A588 STEEL.

2. WELDS NOT INDICATED IN DRAWINGS SHALL BE MINIMUM SIZE CONTINUOUS FILLET WELD IN ACCORDANCE WITH AWS D1.1. FILLET WELDS SHALL BE CONTINUOUS, UNLESS NOTED OTHERWISE.

3. PROVIDE FILLET WELDS AT ALL CONTACT JOINTS BETWEEN STEEL MEMBERS SUFFICIENT TO DEVELOP THE ALLOWABLE TENSILE CAPACITY OF THE SMALLER MEMBER AT THE JOINT, UNLESS NOTED OTHERWISE.

4. ALL GROOVE WELDS SHALL BE FULL PENETRATION, UNLESS NOTED OTHERWISE.

5. AUTOMATICALLY END WELD HEADED STUDS AND DEFORMED BARS WHERE INDICATED ON DRAWINGS. STUDS SHALL CONFORM TO ASTM A108.

STEEL JOIST NOTES

1. DESIGN, DETAILING, FABRICATION AND INSTALLATION OF STEEL JOISTS AND BRIDGING SHALL CONFORM TO THE STANDARDS OF THE STEEL JOIST INSTITUTE (SJI).

2. UNLESS NOTED OTHERWISE, DESIGN STEEL ROOF JOISTS FOR FIFTEEN (15) PSF NET UPLIFT NORMAL TO ROOF SURFACE.

3. ATTACH CONCENTRATED LOADS TO STEEL JOISTS AT JOIST PANEL POINTS OR PROVIDE ADDITIONAL CHORD BRACING IN ACCORDANCE WITH THE CONTRACT DOCUMENTS.

4. WHERE JOIST BOTTOM CHORD EXTENSIONS ARE REQUIRED, DO NOT ATTACH TO COLUMNS, BEAMS OR WALLS, UNLESS NOTED OTHERWISE.

STEEL DECK NOTES

1. ALL STEEL DECK DETAILING, FABRICATION AND INSTALLATION SHALL CONFORM TO THE STANDARDS OF THE STEEL DECK INSTITUTE (SDI).

2. STEEL DECK SHALL BE INSTALLED CONTINUOUSLY ACROSS THREE OR MORE SPANS. DECKING SHALL BE ATTACHED TO STRUCTURAL MEMBERS IMMEDIATELY AFTER ALIGNMENT.

3. ROOF DECK SHALL BE 1.5B, 22 GAGE GALVANIZE STEEL DECK AS MANUFACTURED BY VULCRAFT OR APPROVED SUBSTITUTE. UNLESS NOTED OTHERWISE, FASTEN DECK TO SUPPORTING MEMBERS WITH 5/8 INCH PUDDLE WELDS IN A 36/4 PATTERN WITH THREE (3) #10 TEK SCREW SIDELAP FASTENERS PER SPAN TO RESIST A NET UPLIFT OF FIFTEEN (15) PSF AND A MINIMUM DIAPHRAGM SHEAR VALUE OF 280 PLF.

4. NON-COMPOSITE DECK AT MEZZANINE SHALL BE 1.0C, 22 GAGE GALVANIZED STEEL DECK AS MANUFACTURED BY VULCRAFT OR APPROVED SUBSTITUTE. UNLESS NOTED OTHERWISE, FASTEN DECK TO SUPPORTING MEMBERS WITH WELDS AND WELDING WASHERS IN A 33/4 PATTERN WITH TWO (2) #10 TEK SCREW SIDELAP FASTENERS PER SPAN TO RESIST A MINIMUM DIAPHRAGM SHEAR VALUE OF 1,350 PLF AND NET UPLIFT OF 75 PSF.

PRE-ENGINEERED METAL BUILDING NOTES

1. THE PRE-ENGINEERED METAL BUILDING SYSTEM INCLUDING RIGID FRAMES, COLUMNS, BEAMS, PURLINS, GIRTS, SIDING, ROOFING AND LATERAL BRACING SHALL BE DESIGNED, DETAILED AND FABRICATED BY A MANUFACTURER WHO IS A MEMBER OF THE METAL BUILDNG MANUFACTURER'S ASSOCIATION (MBMA) IN ACCORDANCE WITH THE ABOVE REFERENCED BUILDING CODE AND THE MBMA "LOW RISE BUIDING SYSTEMS MANUAL."

2. THE PRE-ENGINEERED METAL BUILDING SYSTEM INCLUDING RIGID FRAMES, COLUMNS, BEAMS, PURLINS, GIRTS, SIDING, ROOFING AND LATERAL BRACING SHALL BE INSTALLED BY AN EXPERIENCED METAL BUILDING ERECTOR.

3. ALL STRUCTURAL STEEL COMPONENTS AND CONNECTIONS SHALL BE DESIGNED, DETAILED AND FABRICATED BY THE MANUFACTURER IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE AMERICAN INSTITUTE OF STEEL CONSTRUCTION (AISC).

4. ALL COLD FORMED STEEL COMPONENTS AND CONNECTIONS SHALL BE DESIGNED, DETAILED AND FABRICATED BY THE MANUFACTURER IN ACCORDANCE WITH THE SPECIFICATION FOR THE DESIGN OF COLD-FORMED STEEL STRUCTURAL MEMBERS BY THE AMERICAN IRON AND STEEL INSTITUTE (AISI).

5. THE PRE-ENGINEERED METAL BUILDING SYSTEM SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT ALL CODE PRESCRIBED LOADS AND LOAD COMBINATIONS INCLUDING DEAD, LIVE, ROOF LIVE, SNOW, WIND AND SEISMIC LOADS.

6. AS A MINIMUM, ALL METAL BUILDING ROOF FRAMING INCLUDING PURLINS, RIGID FRAMES, COLUMNS AND BEAMS SHALL BE DESIGNED BY THE MANUFACTURER TO SUPPORT A SUPERIMPOSED LOAD OF TWENTY-FIVE (25) PSF (COMPOSED OF FIVE (5) PSF COLLATERAL DEAD LOAD AND TWENTY (20) PSF LIVE LOAD) IN ADDITION TO THE WEIGHT OF THE STRUCTURE.

7. RIGID FRAMES SHALL CONSIST OF WELDED STRUCTURAL STEEL PLATE RAFTERS SUPPORTED BY WELDED STRUCTURAL STEEL FRAME COLUMNS. REFER TO ARCHITECTURAL DRAWINGS FOR COLUMN SIZE AND PROFILE REQUIREMENTS. RIGID FRAMES AND LATERAL BRACING INCLUDING PORTAL FRAMES AND ROD X-BRACING SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM HORIZONTAL STORY DRIFT OF H/120 FOR WIND LOADS.

8. ENDWALL FRAMING SHALL CONSIST OF COLD-FORMED LIGHT GAGE STEEL BEARING FRAME RAFTERS SUPPORTED BY COLD-FORMED LIGHT GAGE STEEL ENDWALL COLUMNS. ENDWALL COLUMNS SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM HORIZONTAL DEFLECTION OF L/120 FOR WIND LOADS. BUILDING X-BRACING IN THE ENDWALL SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM HORIZONTAL STORY DRIFT OF H/120 FOR WIND LOADS.

9. UNLESS NOTED OTHERWISE, EAVE STRUTS SHALL BE COLD-FORMED LIGHT GAGE STEEL "C" SHAPES WITH BRACING AND TIE/SAG RODS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER. EAVE STRUTS SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM VERTICAL DEFLECTION OF L/180 FOR TOTAL LOAD AND L/240 FOR ROOF LIVE LOAD. EAVE STRUTS SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM HORIZONTAL DEFLECTION OF L/120 FOR WIND LOADS.

10. UNLESS NOTED OTHERWISE, WALL GIRTS SHALL BE COLD-FORMED LIGHT GAGE STEEL "Z" SHAPES WITH BRACING AND SAG RODS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER. WALL GIRTS SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM HORIZONTAL DEFLECTION OF L/120 FOR WIND LOADS.

11. UNLESS NOTED OTHERWISE, ROOF PURLINS SHALL BE COLD-FORMED LIGHT GAGE STEEL "Z" SHAPES WITH BRACING AND TIE/SAG RODS IN ACCORDANCE WITH THE RECOMMENDATIONS OF THE MANUFACTURER. ROOF PURLINS SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM VERTICAL DEFLECTION OF L/180 FOR TOTAL LOAD AND L/240 FOR ROOF LIVE LOAD.

12. ALL BOLT HOLES FOR FRAME CONNECTIONS, SPLICES AND THE ATTACHMENT OF SECONDARY MEMBERS SHALL BE SHOP FABRICATED, UNLESS NOTED OTHERWISE.

13. ALL PRE-ENGINEERED METAL BUILDING COMPONENTS SHALL BE COATED WITH PRIMER PAINT IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS.

14. THE CONTRACTOR SHALL SUBMIT COMPLETE ERECTION AND FABRICATION DRAWINGS AND DESIGN CALCULATIONS SEALED BY A PROFESSIONAL ENGINEER REGISTERED IN THE STATE OF THE PROJECT. SUBMITTAL SHALL INCLUDE FRAMING PLANS SHOWING ALL PRE-ENGINEERED METAL BUILDING MEMBERS WITH MARK NUMBERS FOR EACH MEMBER TYPE.

LIGHTGAGE STEEL STUD FRAMING NOTES

1. LIGHTGAGE STEEL FRAMING SHALL BE MANUFACTURED BY DEITRICH INDUSTRIES, OR APPROVED SUBSTITUTE.

2. DESIGN, DETAILING, FABRICATION AND INSTALLATION OF LIGHTGAGE STEEL FRAMING SHALL COMPLY WITH THE MANUFACTURER'S RECOMMENDATIONS.

3. LIGHTGAGE STEEL FRAMING SHALL BE FORMED FROM STEEL HAVING A GALVANIZED COATING MEETING THE REQUIREMENTS OF ASTM A653.

4. ALL EXTERIOR WALL STUDS THAT PROVIDE BACKUP TO MASONRY VENEER SHALL BE DESIGNED BY THE MANUFACTURER FOR A MAXIMUM DEFLECTION OF L/600 UNDER CODE PRESCRIBED LATERAL LOADS.

5. AS A MINIMUM, EXTERIOR WALL STUDS SHALL BE SIX (6) INCH CSJ, 18 GAGE STEEL STUDS AT SIXTEEN (16) INCHES ON CENTER, UNLESS NOTED OTHERWISE.

6. AS A MINIMUM, CONTINUOUS TOP AND BOTTOM TRACK FOR EXTERIOR WALLS SHALL BE SIX (6) INCH TSC, 20 GAGE STEEL TRACK. UNLESS NOTED OTHERWISE, FASTEN TRACK TO EACH STUD WITH #8 TEK SCREWS AT EACH SIDE OF WALL.

7. AS A MINIMUM, THE BOTTOM TRACK OF EXTERIOR WALLS SHALL BE FASTENED TO THE FOUNDATION WITH 0.177 INCH DIAMETER POWDER ACTUATED FASTENERS, WITH A MINIMUM OF 1 7/16 INCHES EMBEDMENT, AT TWENTY-FOUR (24) INCHES ON CENTER.

REINFORCED CONCRETE MASONRY NOTES

1. REINFORCED CONCRETE MASONRY WALL CONSTRUCTION HAS BEEN DESIGNED FOR A MINIMUM COMPRESSION STRENGTH (f'm) OF 1,500 PSI. THIS VALUE SHALL BE VERIFIED IN ACCORDANCE WITH NCMA TR 75B, "SPECIFICATIONS FOR THE DESIGN AND CONSTRUCTION OF LOAD-BEARING CONCRETE MASONRY."

2. CONCRETE BLOCK SHALL BE ASTM C90 LIGHT-WEIGHT UNITS OF EIGHT (8) INCH NOMINAL THICKNESS WITH A MINIMUM COMPRESSIVE STRENGTH OF 1,900 PSI ON THE NET AREA OF THE BLOCK.

3. MORTAR SHALL BE TYPE "M" OR "S" IN ACCORDANCE WITH ASTM C270 AND SHALL HAVE A TWENTY-EIGHT (28) DAY COMPRESSIVE STRENGTH OF 2,500 PSI OR 1,800 PSI, RESPECTIVELY. AGGREGATES FOR MORTAR SHALL CONFORM TO ASTM C144.

4. GROUT SHALL CONFORM TO ASTM C476 WITH A MAXIMUM AGGREGATE SIZE OF 3/8 INCH AND A 28-DAY COMPRESSIVE STRENGTH OF 2,000 PSI. AGGREGATES FOR GROUT SHALL CONFORM TO ASTM C404.

5. LAP SPLICE LENGTH FOR CONTINUOUS DEFORMED BAR REINFORCEMENT IN CONCRETE MASONRY CONSTRUCTION SHALL BE AS FOLLOWS:

#3 BARS	19 INCHES MINIMUM
#4 BARS	25 INCHES MINIMUM
#5 BARS	31 INCHES MINIMUM
#6 BARS	57 INCHES MINIMUM

6. ALL CELLS CONTAINING REINFORCING BARS, BOLTS OR OTHER METAL FABRICATIONS SHALL BE GROUTED SOLID. ANY CELLS AT OR BELOW FINISHED GRADE SHALL BE GROUTED SOLID.

7. REINFORCED CONCRETE MASONRY CONSTRUCTION SHALL BE RUNNING BOND, UNLESS NOTED OTHERWISE.

STRUCTURAL ABBREVIATIONS:

THE FOLLOWING ABBREVIATIONS ARE REFERENCED IN THE STRUCTURAL DRAWINGS. PLEASE CONTACT THE STRUCTURAL ENGINEER OF RECORD FOR ANY CLARIFICATION, PRIOR TO FABRICATION.

ACI	AMERICAN CONCRETE INSTITUTE ADDL ADDITIONAL	LBS	POUNDS
		LL	LIVE LOAD
AISC	AMERICAN INSTITUTE OF STEEL CONSTRUCTION	LLH	LONG LEG HORIZONTAL
		LLV	LONG LEG VERTICAL
AISI	AMERICAN IRON AND STEEL INSTITUTE	LSL	LAMINATED STRAND LUMBER
		LT. GAGE	LIGHT GAGE
ALT.	ALTERNATE	LVL	LAMINATED VENEER LUMBER
APA	AMERICAN PLYWOOD ASSOCIATION (ENGINEERED WOOD ASSOCIATION)	MAT'L	MATERIAL
		MAX.	MAXIMUM
ARCH'L	ARCHITECTURAL, ARCHITECT	MECH'L	MECHANICAL
ASS'Y	ASSEMBLY	MFR.	MANUFACTURER
ASTM	ASTM INTERNATIONAL (FORMERLY AMERICAN SOCIETY FOR TESTING AND MATERIALS)	MIN.	MINIMUM
		NDS	NATIONAL DESIGN SPECIFICATION FOR WOOD CONSTRUCTION
B/	BOTTOM OF	NTS	NOT TO SCALE
B/BEAM	BOTTOM OF BEAM		
BLDG.	BUILDING	O.C.	ON CENTER
BOT.	BOTTOM	O.D.	OUTSIDE DIAMETER
BRG.	BEARING	OPNG.	OPENING
C.L.	CENTERLINE	OPP.	OPPOSITE
CLG.	CEILING		
CMU	CONCRETE MASONRY UNIT(S)	P.E.M.B.	PRE-ENGINEERED METAL BUILDING
COL.	COLUMN	PL	PLATE
CONC.	CONCRETE	PLF	POUNDS PER LINEAR FOOT
CONN.	CONNECTION	PREFAB.	PRE-FABRICATED
CONT.	CONTINUOUS	PSF	POUNDS PER SQUARE FOOT
CONST.	CONSTRUCTION	PSI	POUNDS PER SQUARE INCH
COORD.	COORDINATE	PSL	PARALLEL STRAND LUMBER
CRSI	CONCRETE REINFORCING STEEL INSTITUTE	P.T.	POST-TENSIONED
		R	REMAINING
DB	DROP BEAM	REF.	REFERENCE
D.B.A.	DEFORMED BAR ANCHOR	REINF.	REINFORCE, REINFORCED
DFL	DOUGLAS FIR/LARCH	REQ'D	REQUIRED
DIA.	DIAMETER	REV.	REVISION
DIM.	DIMENSION	RTU	ROOF TOP UNIT
DL	DEAD LOAD		
DTL	DETAIL	SCHED.	SCHEDULE(D)
DWL	DOWEL	SIM.	SIMILAR
		SJI	STEEL JOIST INSTITUTE
EL.	ELEVATION	SPA.	SPACE(S), SPACED
EMBED.	EMBEDMENT	SQ.	SQUARE
EQ.	EQUAL	STD.	STANDARD
EXP.	EXPANSION	STIRR.	STIRRUP(S)
		SYP	SOUTHERN PINE
FB	FLUSH BEAM		
F/	FLOOR FINISHED FLOOR	T/	TOP OF
FLR.	FLOOR	T/CONC.	TOP OF CONC
FTG.	FOOTING	T/FOOTING	TOP OF FOOTING
		T/METAL	TOP OF METAL
GALV.	GALVANIZED	T/PANEL	TOP OF PANEL
GYP.	GYPSUM	T/PARAPET	TOP OF PARAPET
		T/PIER	TOP OF PIER
HORIZ.	HORIZONTAL	T/PILECAP	TOP OF PILECAP
HVAC	HEATING, VENTILATION AND AIR CONDITIONING	T/SHEATHING	TOP OF SHEATHING
		T/SLAB	TOP OF SLAB
		T/STEEL	TOP OF STEEL
IBC	INTERNATIONAL BUILDING CODE	T/WALL	TOP OF WALL
		TYP.	TYPICAL
I.D.	INSIDE DIAMETER		
INFO.	INFORMATION	U.N.O.	UNLESS NOTED OTHERWISE
		VERT.	VERTICAL
KIP	KILOPOUND (1,000 POUNDS)		
KSI	KILOPOUNDS PER SQUARE INCH	W/	WITH
		W/O	WITHOUT
		W.W.F.	WELDED WIRE FABRIC

CLINTON W. FARRIS #28983
DATE SIGNED: 7-28-22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS

CONCRETE REINFORCING LAP SPLICE SCHEDULE

BAR SIZE	LAP
3	1'-6"
4	2'-0"
5	2'-6"
6	3'-0"
7	4'-2"
8	4'-8"
9	5'-4"
10	6'-0"
11	6'-8"

CONCRETE DOWEL SCHEDULE

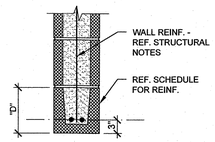
MARK	SIZE	A	B	C
DWL. A	#4	2'-6"	1'-0"	-
DWL. B	#5	2'-9"	0'-8"	-
DWL. C	#3	2'-0"	2'-0"	-
DWL. D	#5	2'-0"	1'-0"	-
DWL. E	#4	2'-0"	AS REQ'D	-
DWL. F	#4	AS REQ'D	0'-8"	-
DWL. G	#4	2'-6"	0'-8"	0'-8"

NOTES:

1. SCHEDULED DOWELS ARE MARKED "DWL." ON THE SECTIONS AND DETAILS.

2. DOWEL SPACING TO BE THE SAME AS VERTICAL BEAM OR WALL REINFORCEMENT, UNLESS NOTED OTHERWISE.

3. STRAIGHT BARS SHALL BE PLACED WITH ONE HALF OF BAR LENGTH ON EACH SIDE OF COLD JOINT, UNLESS NOTED OTHERWISE.



TYPE "A" - CMU LINTEL BLOCK

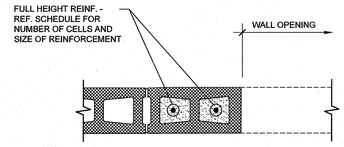
CONCRETE MASONRY LINTEL SCHEDULE

MARK	TYPE	DEPTH, "D"	REINF.	MAXIMUM OPENING
TYPICAL, U.N.O.	A	8"	2-#4 CONT.	4'-0"
TYPICAL, U.N.O.	A	16"	2-#5 CONT.	8'-0"
TYPICAL, U.N.O.	A	24"	2-#5 CONT.	12'-0"

NOTES:

1. EXTEND BEAMS 8" BEYOND FACE OF OPENING.

2. VERTICAL CELLS ADJACENT TO OPENINGS SHALL BE REINFORCED PER TYP. WALL REINF. AND GROUTED SOLID - REF. STRUCTURAL NOTES.



PLAN

JAMB SCHEDULE

OPENING WIDTH	CMU JAMB			
	EXTERIOR		INTERIOR	
	NUMBER OF GROUTED CELLS	REINF./CELL	NUMBER OF GROUTED CELLS	REINF./CELL
≤ 4'-0"	2	1-#5	2	1-#5
≤ 8'-0"	3	1-#5	2	1-#5
≤ 12'-0"	3	1-#6	2	1-#5

NOTES:

1. HORIZONTAL REINFORCEMENT OMITTED FOR CLARITY. REFER TO 02MTYP02 FOR ADDITIONAL INFORMATION.

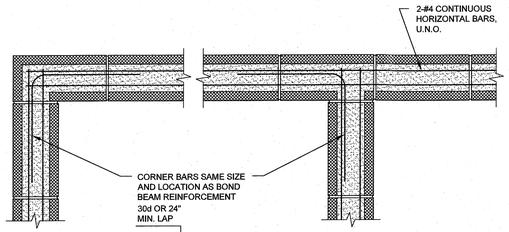
2. AT MEZZANINES, USE FOUR (4) GROUTED CELLS WITH 2-#8 BARS (1 EACH FACE) IN EACH CELL (4'-0" OPENING MAX.)

01 **SCHEDULE**
NO SCALE

02 **SCHEDULE**
NO SCALE

03 **TYPICAL CMU LINTEL SCHEDULE**
NO SCALE

04 **TYPICAL CMU JAMB SCHEDULE**
NO SCALE

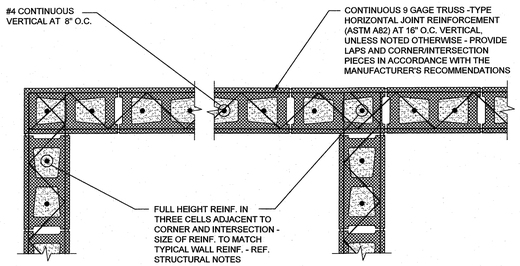


NOTES:
1. REFER TO 6/S102 FOR ADDITIONAL INFORMATION.

2. UNLESS NOTED OTHERWISE, PROVIDE CONTINUOUS 8" DEEP BOND BEAM AT THE TOP AND AT 8'-0", 16'-0", AND 24'-0" ABOVE FINISHED FLOOR IN ALL WALLS.

PLAN

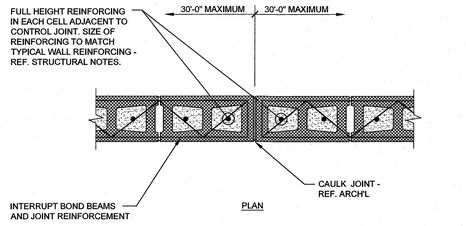
TYPICAL CMU WALL BOND BEAM REINFORCEMENT DETAIL
05 NO SCALE



PLAN

NOTE: REF. REINFORCED CONCRETE MASONRY NOTES FOR ADDITIONAL INFORMATION.

TYPICAL CMU WALL REINFORCEMENT DETAIL
06 NO SCALE



PLAN

NOTES:
1. REF. 6/S102 FOR ADDITIONAL INFORMATION.

2. CONTROL JOINTS SHALL BE LOCATED AT WALL OPENINGS, AT 30'-0" O.C. MAXIMUM, AND AS SHOWN IN THE ARCHITECTURAL DRAWINGS.

3. OFFSET CONTROL JOINTS TO THE END LINTEL AND SUPPORTING CELLS AT OPENINGS.

TYPICAL CMU WALL CONTROL JOINT DETAIL
07 NO SCALE

TYPICAL STEEL LINTEL SCHEDULE FOR OPENINGS IN MASONRY VENEER

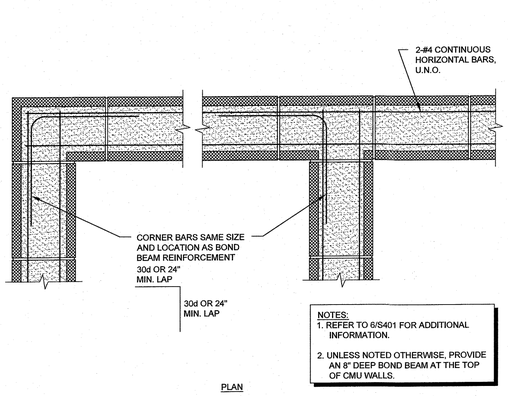
MAXIMUM SPAN	SIZE	MINIMUM BEARING
3'-0"	L3 1/2x3 1/2x3/8	8"
5'-0"	L4x4x3/8	8"
8'-0"	L5x3 1/2x3/8 LLV	12"
10'-0"	L6x4x3/8 LLV	12"

NOTES:

1. CONTACT ENGINEER OF RECORD FOR OPENINGS LARGER THAN 10'-0"

2. ALL LINTELS IN EXTERIOR WALLS SHALL BE PROTECTED WITH PAINT OR HOT DIP GALVANIZING CONFORMING TO ASTM A123.

08 **SCHEDULE**
NO SCALE

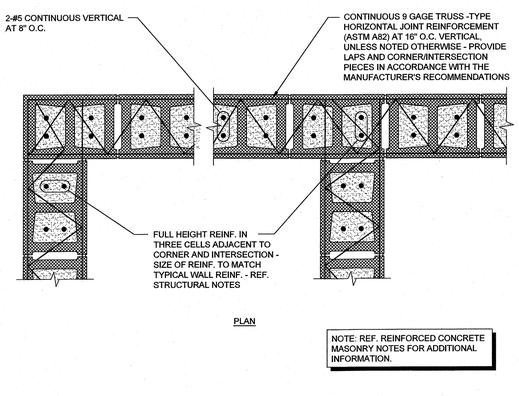


NOTES:
1. REFER TO 6/S401 FOR ADDITIONAL INFORMATION.

2. UNLESS NOTED OTHERWISE, PROVIDE AN 8" DEEP BOND BEAM AT THE TOP OF CMU WALLS.

PLAN

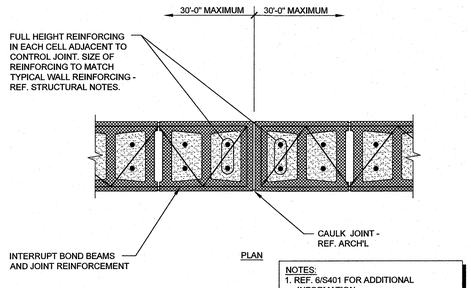
TYPICAL 12" CMU WALL BOND BEAM REINFORCEMENT DETAIL AT MEZZANINE
09 NO SCALE



PLAN

NOTE: REF. REINFORCED CONCRETE MASONRY NOTES FOR ADDITIONAL INFORMATION.

TYPICAL 12" CMU WALL REINFORCEMENT DETAIL AT MEZZANINE
10 NO SCALE



PLAN

NOTES:
1. REF. 6/S401 FOR ADDITIONAL INFORMATION.

2. CONTROL JOINTS SHALL BE LOCATED AT WALL OPENINGS, AT 30'-0" O.C. MAXIMUM, AND AS SHOWN IN THE ARCHITECTURAL DRAWINGS.

3. OFFSET CONTROL JOINTS TO THE END LINTEL AND SUPPORTING CELLS AT OPENINGS.

TYPICAL 12" CMU WALL CONTROL JOINT DETAIL AT MEZZANINE
11 NO SCALE

REGISTERED ARCHITECT
CLINTON W. FARRIS
3998
STATE OF TEXAS
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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS

FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S102

STATE OF TEXAS
R. TRENT PERKINS
84264
REGISTERED PROFESSIONAL ENGINEER
7/28/2022
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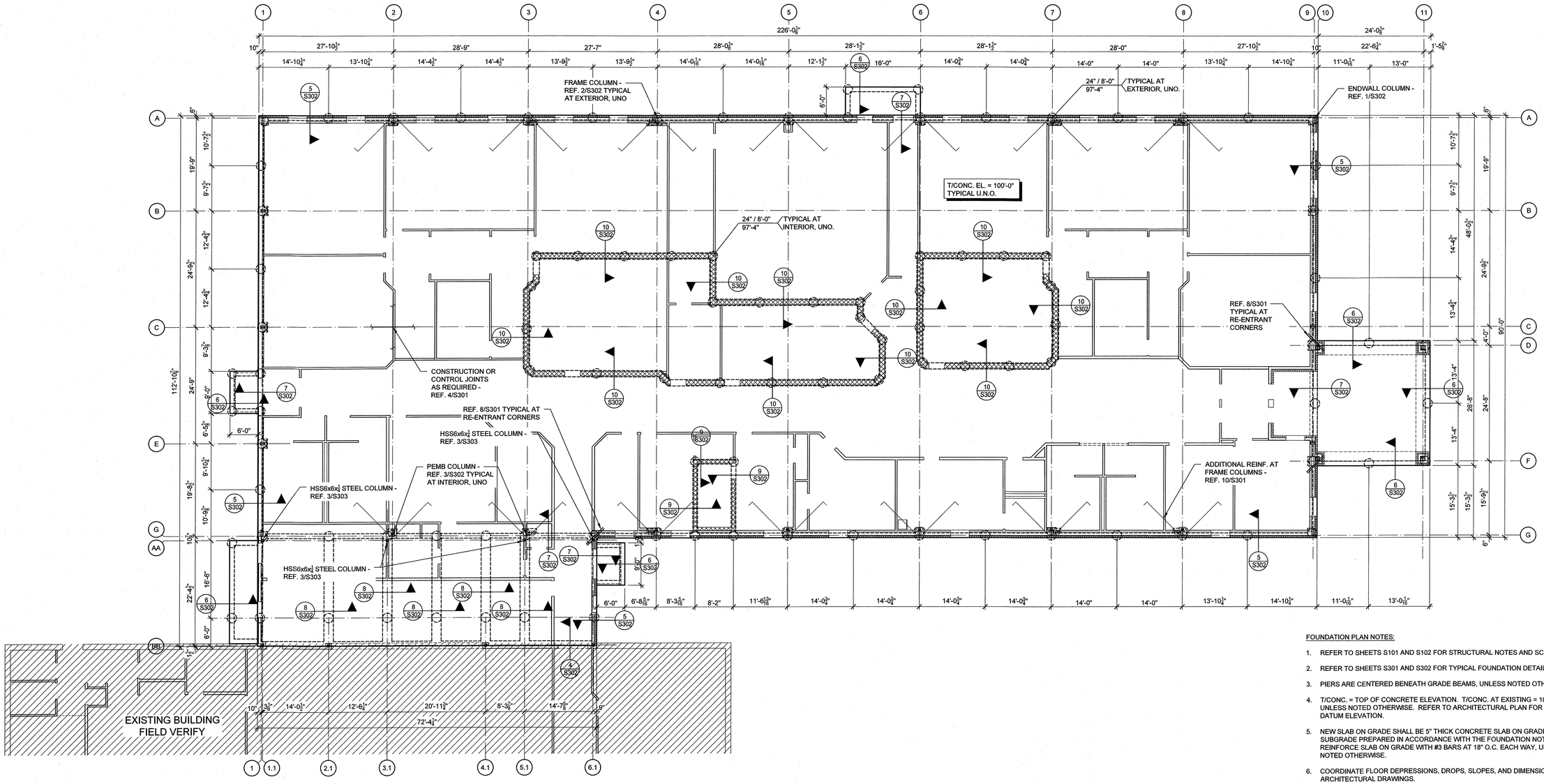
RTP#: 22062



FOUNDATION PLAN - SECONDARY

SCALE: 3/32"=1'-0"

NORTH

FOUNDATION PLAN NOTES:

1. REFER TO SHEETS S101 AND S102 FOR STRUCTURAL NOTES AND SCHEDULES.
2. REFER TO SHEETS S301 AND S302 FOR TYPICAL FOUNDATION DETAILS.
3. PIERS ARE CENTERED BENEATH GRADE BEAMS, UNLESS NOTED OTHERWISE.
4. T/CONC. = TOP OF CONCRETE ELEVATION. T/CONC. AT EXISTING = 100'-0", UNLESS NOTED OTHERWISE. REFER TO ARCHITECTURAL PLAN FOR RELATIVE DATUM ELEVATION.
5. NEW SLAB ON GRADE SHALL BE 5" THICK CONCRETE SLAB ON GRADE OVER SUBGRADE PREPARED IN ACCORDANCE WITH THE FOUNDATION NOTES. REINFORCE SLAB ON GRADE WITH #3 BARS AT 18" O.C. EACH WAY, UNLESS NOTED OTHERWISE.
6. COORDINATE FLOOR DEPRESSIONS, DROPS, SLOPES, AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
7. PIERS ARE NOTED THUS ON PLAN (REF. 1/S301) :

PIER DIA. / PENETRATION
TOP OF PIER ELEVATION

NOTE:
FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION AND/OR INSTALLATION OF NEW STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

CLINTON W. FARRIS #28983
DATE SIGNED: 7-28-22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET
GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S201

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LICENSED PROFESSIONAL ENGINEER
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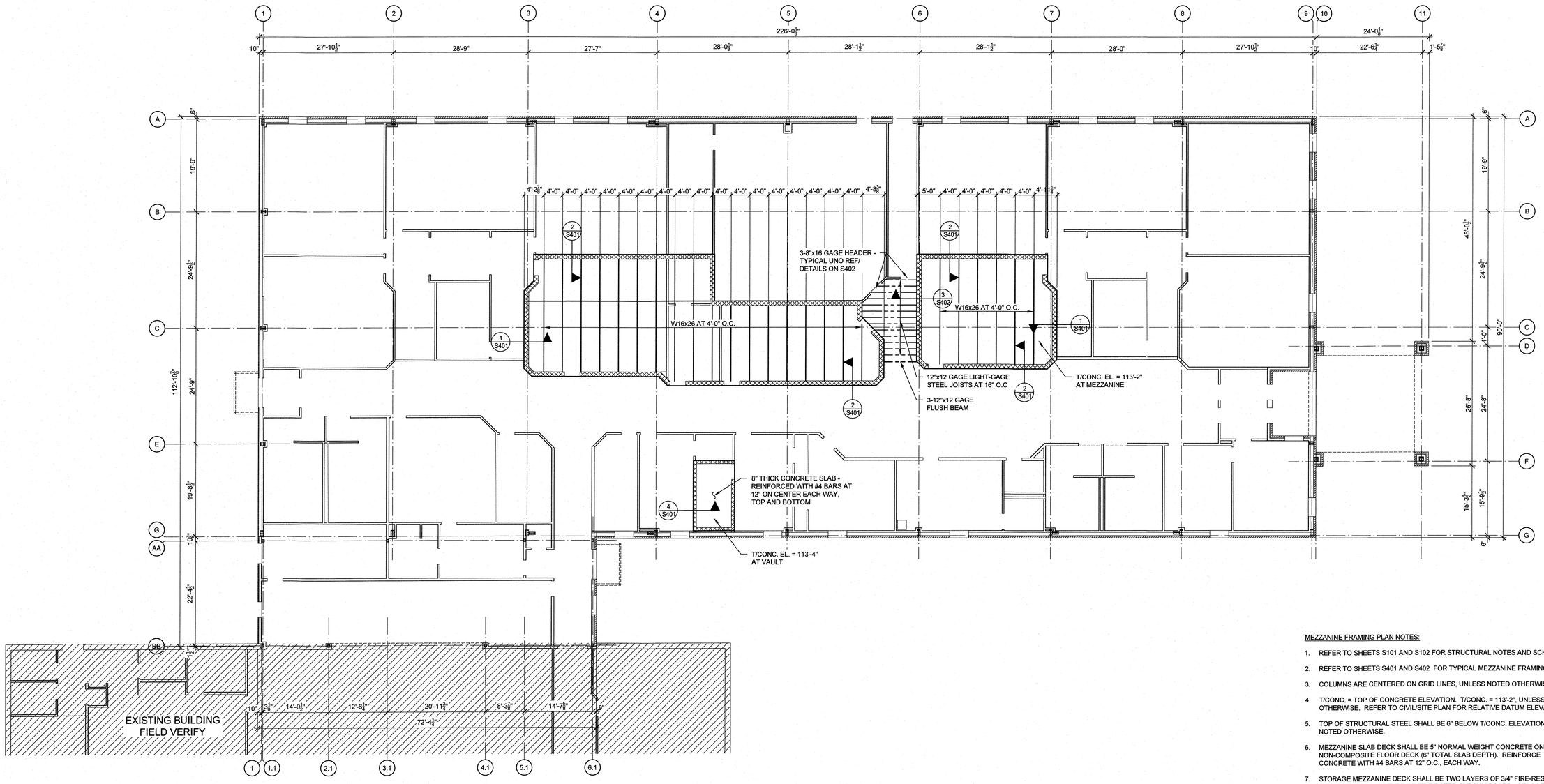

MEZZANINE FRAMING PLAN - SECONDARY

SCALE: 3/32"=1'-0"

NORTH

NOTE:
FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION AND/OR INSTALLATION OF NEW STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

MEZZANINE FRAMING PLAN NOTES:

1. REFER TO SHEETS S101 AND S102 FOR STRUCTURAL NOTES AND SCHEDULES.
2. REFER TO SHEETS S401 AND S402 FOR TYPICAL MEZZANINE FRAMING DETAILS.
3. COLUMNS ARE CENTERED ON GRID LINES, UNLESS NOTED OTHERWISE.
4. T/CONC. = TOP OF CONCRETE ELEVATION. T/CONC. = 113'-2", UNLESS NOTED OTHERWISE. REFER TO CIVIL/SITE PLAN FOR RELATIVE DATUM ELEVATION.
5. TOP OF STRUCTURAL STEEL SHALL BE 6" BELOW T/CONC. ELEVATION, UNLESS NOTED OTHERWISE.
6. MEZZANINE SLAB DECK SHALL BE 5" NORMAL WEIGHT CONCRETE ON NON-COMPOSITE FLOOR DECK (6" TOTAL SLAB DEPTH). REINFORCE CONCRETE WITH #4 BARS AT 12" O.C., EACH WAY.
7. STORAGE MEZZANINE DECK SHALL BE TWO LAYERS OF 3/4" FIRE-RESISTANT PLYWOOD OVER 5/8" TYPE "X" FIRE-RATED GYPSUM BOARD. STAGGER ALL PANEL EDGES BETWEEN ADJACENT LAYERS OF SHEATHING. FASTEN ALL PANELS WITH #10 TEK SCREWS AT SIX (6) INCHES ON CENTER AT ALL PANEL EDGES AND INTERMEDIATE SUPPORTS.
8. COORDINATE FLOOR DEPRESSIONS, DROPS, SLOPES, AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.

REGISTERED ARCHITECT CLINTON W. FARRIS 28983 STATE OF TEXAS
CLINTON W. FARRIS #28983
DATE SIGNED: 7.28.22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO. 20864.00

SHEET NO. S202

STATE OF TEXAS R. TRENT PERKINS 84264 LICENSED PROFESSIONAL ENGINEER
7/28/2022
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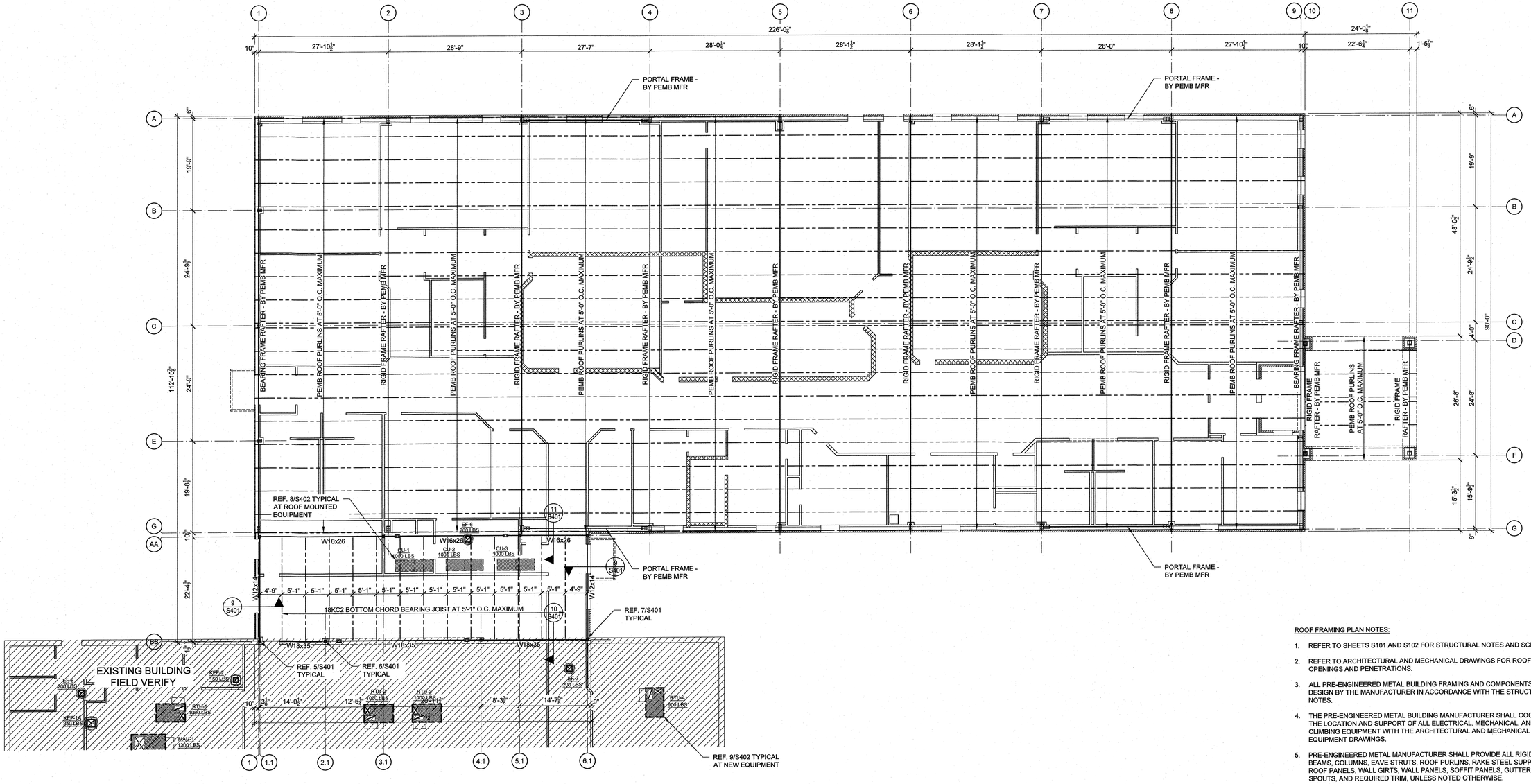

ROOF FRAMING PLAN - SECONDARY

SCALE: 3/32"=1'-0"

NORTH

ROOF FRAMING PLAN NOTES:

1. REFER TO SHEETS S101 AND S102 FOR STRUCTURAL NOTES AND SCHEDULES.
2. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ROOF OPENINGS AND PENETRATIONS.
3. ALL PRE-ENGINEERED METAL BUILDING FRAMING AND COMPONENTS SHALL BE DESIGN BY THE MANUFACTURER IN ACCORDANCE WITH THE STRUCTURAL NOTES.
4. THE PRE-ENGINEERED METAL BUILDING MANUFACTURER SHALL COORDINATE THE LOCATION AND SUPPORT OF ALL ELECTRICAL, MECHANICAL, AND CLIMBING EQUIPMENT WITH THE ARCHITECTURAL AND MECHANICAL EQUIPMENT DRAWINGS.
5. PRE-ENGINEERED METAL MANUFACTURER SHALL PROVIDE ALL RIGID FRAMES, BEAMS, COLUMNS, EAVE STRUTS, ROOF PURLINS, RAKE STEEL SUPPORTS, ROOF PANELS, WALL GIRTS, WALL PANELS, SOFFIT PANELS, GUTTERS, DOWN SPOUTS, AND REQUIRED TRIM, UNLESS NOTED OTHERWISE.

NOTE:
FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION AND/OR INSTALLATION OF NEW STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

REGISTERED ARCHITECT
CLIFFORD W. FARRIS
28983
STATE OF TEXAS
CLIFFORD W FARRIS #28983
DATE SIGNED: 7.28.22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S203

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REGISTERED PROFESSIONAL ENGINEER
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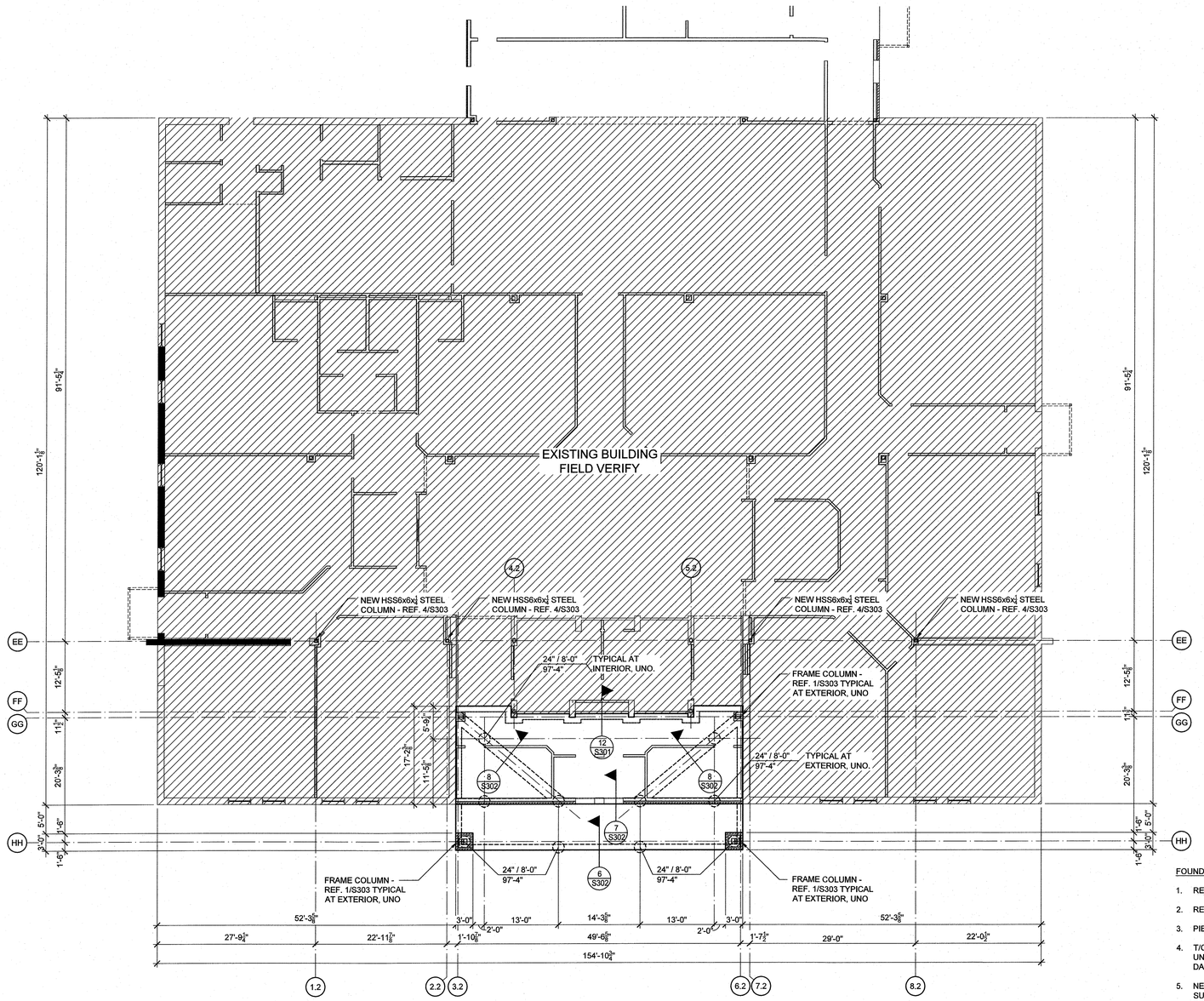

FOUNDATION PLAN - ELEMENTARY

SCALE: 3/32"=1'-0"

NORTH

NOTE:
FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION AND/OR INSTALLATION OF NEW STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

FOUNDATION PLAN NOTES:

1. REFER TO SHEETS S101 AND S102 FOR STRUCTURAL NOTES AND SCHEDULES.
2. REFER TO SHEETS S301 AND S302 FOR TYPICAL FOUNDATION DETAILS.
3. PIERS ARE CENTERED BENEATH GRADE BEAMS, UNLESS NOTED OTHERWISE.
4. T/CONC. = TOP OF CONCRETE ELEVATION. T/CONC. AT EXISTING = 100'-0", UNLESS NOTED OTHERWISE. REFER TO ARCHITECTURAL PLAN FOR RELATIVE DATUM ELEVATION.
5. NEW SLAB ON GRADE SHALL BE 5" THICK CONCRETE SLAB ON GRADE OVER SUBGRADE PREPARED IN ACCORDANCE WITH THE FOUNDATION NOTES. REINFORCE SLAB ON GRADE WITH #3 BARS AT 18" O.C. EACH WAY, UNLESS NOTED OTHERWISE.
6. COORDINATE FLOOR DEPRESSIONS, DROPS, SLOPES, AND DIMENSIONS WITH ARCHITECTURAL DRAWINGS.
7. PIERS ARE NOTED THUS ON PLAN (REF. 1/S301) :

PIER DIA. / PENETRATION
TOP OF PIER ELEVATION

REGISTERED ARCHITECT
CLINTON W. FARRIS
#28983
STATE OF TEXAS
CLINTON W. FARRIS #28983
DATE SIGNED: 7-28-22

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ADDITIONS AND RENOVATIONS TO THE
GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.
GORDON, TEXAS 76453
112 RUSK STREET

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S204

STATE OF TEXAS
R. TRENT PERKINS
84264
REGISTERED
PROFESSIONAL ENGINEER
7/28/2022
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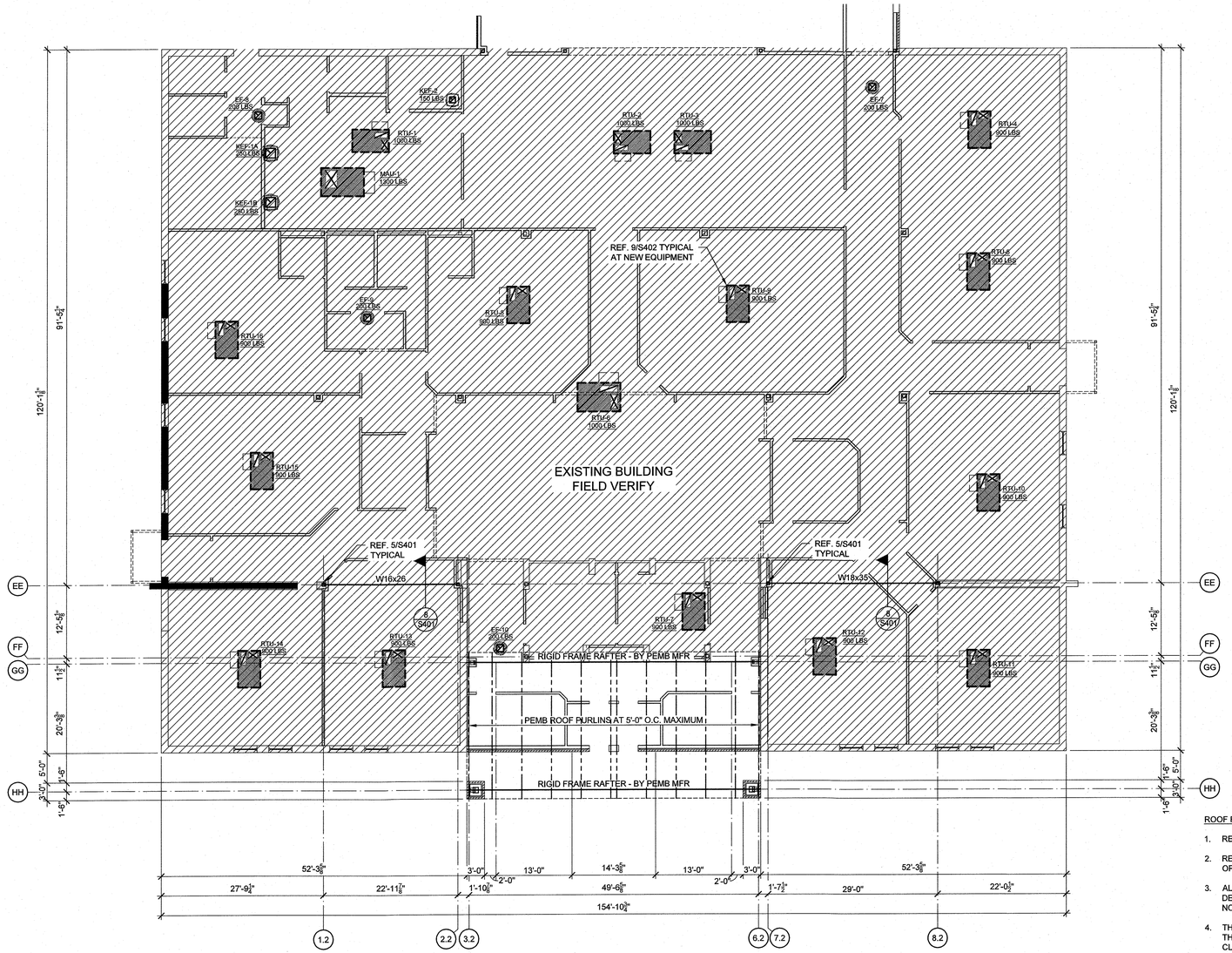
RTP#: 22062



ROOF FRAMING PLAN - ELEMENTARY

SCALE: 3/32"=1'-0"

NORTH

ROOF FRAMING PLAN NOTES:

1. REFER TO SHEETS S101 AND S102 FOR STRUCTURAL NOTES AND SCHEDULES.
2. REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR ROOF OPENINGS AND PENETRATIONS.
3. ALL PRE-ENGINEERED METAL BUILDING FRAMING AND COMPONENTS SHALL BE DESIGN BY THE MANUFACTURER IN ACCORDANCE WITH THE STRUCTURAL NOTES.
4. THE PRE-ENGINEERED METAL BUILDING MANUFACTURER SHALL COORDINATE THE LOCATION AND SUPPORT OF ALL ELECTRICAL, MECHANICAL, AND CLIMBING EQUIPMENT WITH THE ARCHITECTURAL AND MECHANICAL EQUIPMENT DRAWINGS.
5. PRE-ENGINEERED METAL MANUFACTURER SHALL PROVIDE ALL RIGID FRAMES, BEAMS, COLUMNS, EAVE STRUTS, ROOF PURLINS, RAKE STEEL SUPPORTS, ROOF PANELS, WALL GIRTS, WALL PANELS, SOFFIT PANELS, GUTTERS, DOWN SPOUTS, AND REQUIRED TRIM, UNLESS NOTED OTHERWISE.

NOTE:
FIELD VERIFY ALL EXISTING CONDITIONS PRIOR TO FABRICATION AND/OR INSTALLATION OF NEW STRUCTURAL MEMBERS. NOTIFY ARCHITECT OF ANY DISCREPANCIES BETWEEN CONDITIONS OBSERVED IN THE FIELD AND CONDITIONS INDICATED ON PLAN.

REGISTERED ARCHITECT
CLINTON W. FARRIS
STATE OF TEXAS
28983

CLINTON W. FARRIS #28983
DATE SIGNED: 1.28.22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

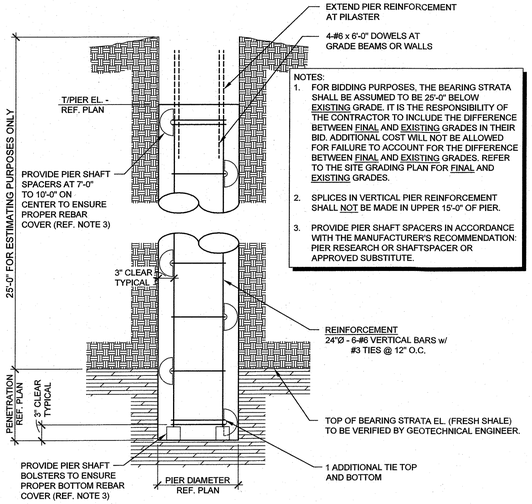
PROJECT NO.
20864.00

SHEET NO.
S205

STATE OF TEXAS
R. TRENT PERKINS
84264
REGISTERED
PROFESSIONAL ENGINEER
7/28/2022

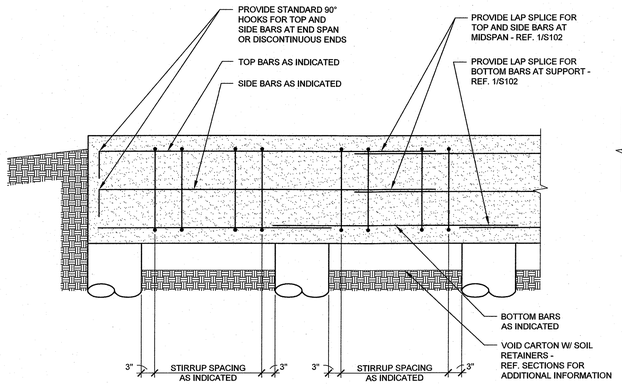
THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY R. TRENT PERKINS, P.E. 84264

STRUCTURAL, PLLC
Texas Engineering Firm F-16159
P.O. Box 1599
Rockwall, Texas 75087
Phone 214-293-2503
RTP#: 22062



NOTES:
1. FOR BIDDING PURPOSES, THE BEARING STRATA SHALL BE ASSUMED TO BE 25'-0" BELOW EXISTING GRADE. IT IS THE RESPONSIBILITY OF THE CONTRACTOR TO INCLUDE THE DIFFERENCE BETWEEN FINAL AND EXISTING GRADES IN THEIR BID. ADDITIONAL COST WILL NOT BE ALLOWED FOR FAILURE TO ACCOUNT FOR THE DIFFERENCE BETWEEN FINAL AND EXISTING GRADES. REFER TO THE SITE GRADING PLAN FOR FINAL AND EXISTING GRADES.
2. SPLICES IN VERTICAL PIER REINFORCEMENT SHALL NOT BE MADE IN UPPER 15'-0" OF PIER.
3. PROVIDE PIER SHAFT SPACERS IN ACCORDANCE WITH THE MANUFACTURER'S RECOMMENDATION: PIER RESEARCH OR SHAFTSPACER OR APPROVED SUBSTITUTE.

01 TYPICAL DRILLED PIER DETAIL
NO SCALE



02 TYPICAL UNSCHEDULED CONCRETE BEAM REINFORCEMENT DETAIL
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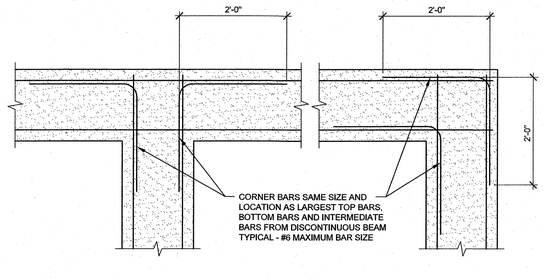
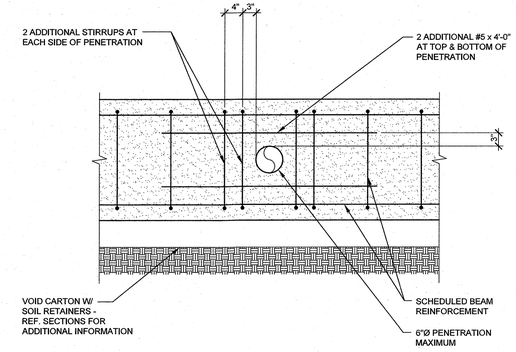


03 TYPICAL CORNER BARS AT CONCRETE WALL, BEAM AND FOOTING INTERSECTION DETAIL
NO SCALE



04 TYPICAL GRADE BEAM PENETRATION DETAIL
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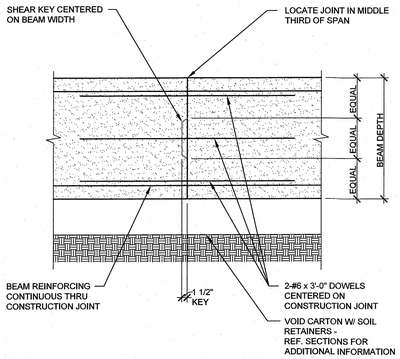

05 TYPICAL GRADE BEAM CONSTRUCTION JOINT DETAIL
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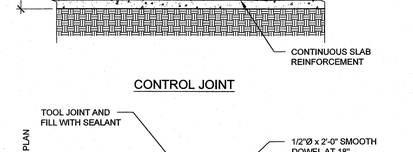
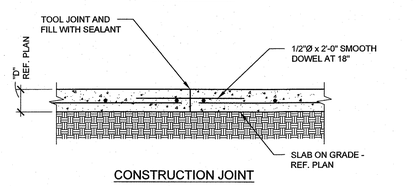


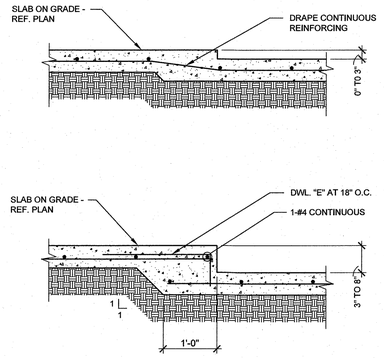
CONTROL JOINT



CONSTRUCTION JOINT

NOTES:
1. CONTROL JOINTS SHALL BE CUT INTO SLAB WITHIN 8 HOURS OF CONCRETE PLACEMENT AT COLUMN CENTERLINES AND/OR 16'-0" O.C. MAXIMUM.
2. CONTRACTOR SHALL PROVIDE A CONTROL/CONSTRUCTION JOINT PLAN FOR APPROVAL PRIOR TO FABRICATION OR INSTALLATION OF MATERIALS.
3. CONTRACTOR SHALL COORDINATE LOCATION OF CONTROL/CONSTRUCTION JOINTS WITH ARCHITECTURAL FLOOR FINISHES.

06 TYPICAL JOINTS IN SLAB-ON-GRADE DETAIL
NO SCALE



07 TYPICAL DROP IN SLAB-ON-GRADE DETAILS
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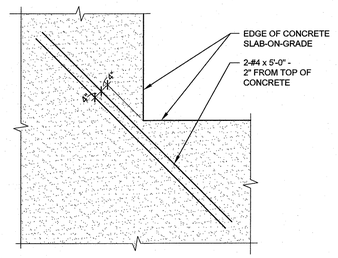

08 TYPICAL RE-ENTRANT SLAB CORNER REINFORCING DETAIL
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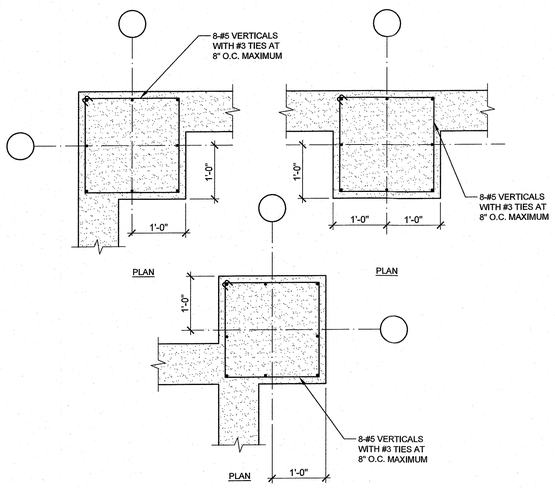

09 TYPICAL PILASTER DETAIL
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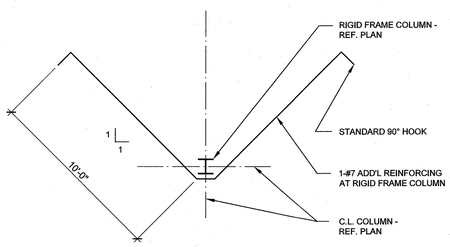

NOTE:
1. PLACE ADD'L STEEL AT MID-DEPTH OF FLOOR SLAB.

10 TYPICAL ADDITIONAL REINFORCEMENT DETAIL
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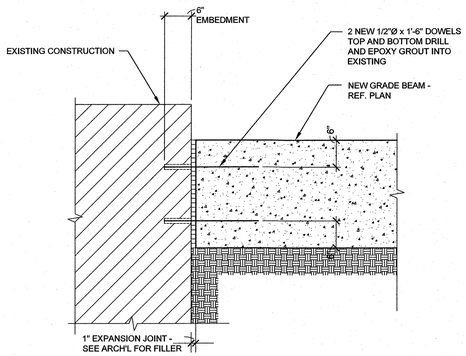
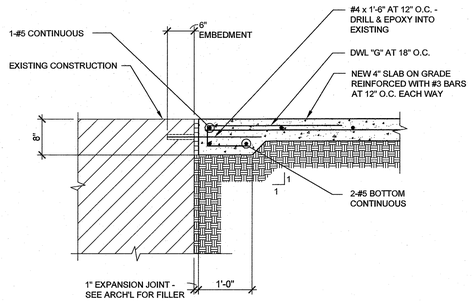


NOTE:
DETAIL IS ONLY FOR DOWEL PLACEMENT. REFER PLANS AND SECTIONS FOR ADDITIONAL INFORMATION.

11 DETAIL
NO SCALE



12 SECTION
NO SCALE

CLINTON W. FARRIS #28983
DATE SIGNED: 7.28.22

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ESTABLISHED 1962
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4724 OLD JACKSBORO HIGHWAY
WICHITA FALLS, TEXAS 76302-3599
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WEB: www.hpa1962.com

ADDITIONS AND RENOVATIONS TO THE
GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET
GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

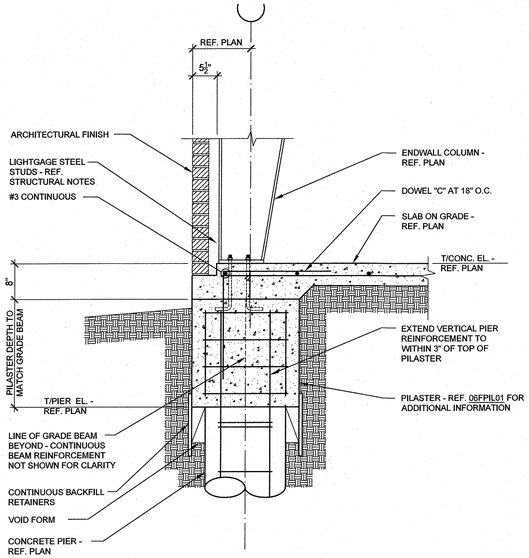
NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S301

THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY R. TRENT PERKINS, P.E. 84264 7/28/2022

R. TRENT PERKINS STRUCTURAL, PLLC
Texas Engineering Firm F-16159
P.O. Box 1599
Rockwall, Texas 75087
Phone 214-293-2503
RTP#: 22062



01 **TYPICAL FOUNDATION DETAIL**
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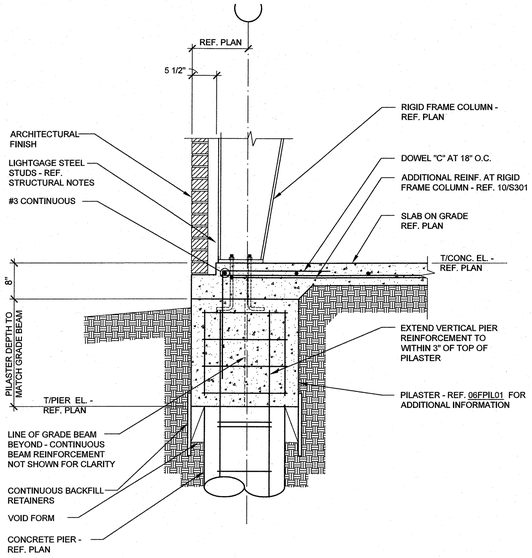

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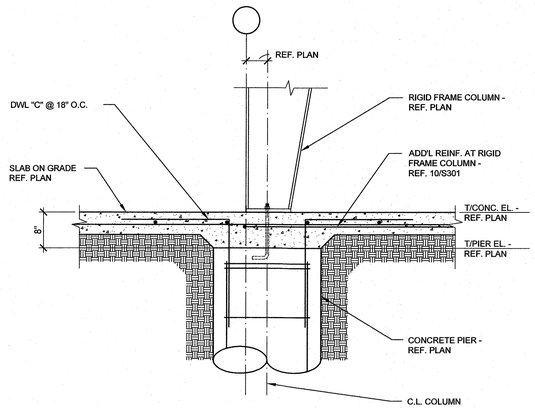

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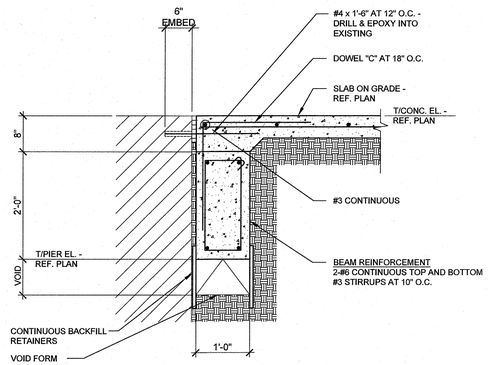

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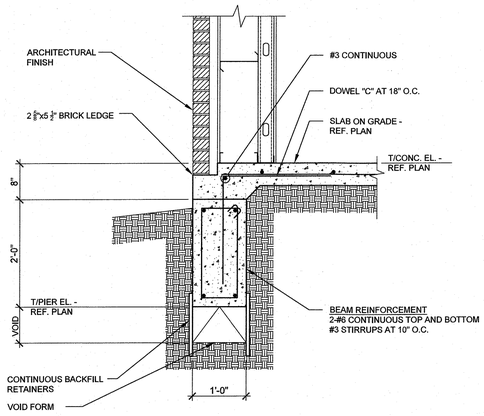

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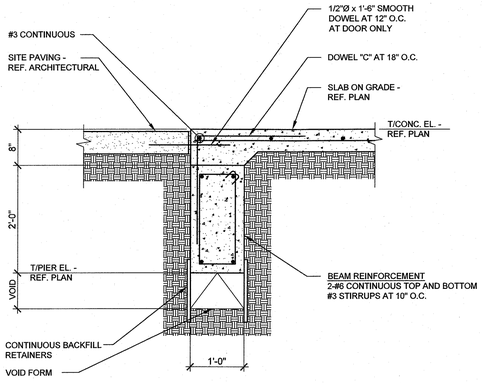

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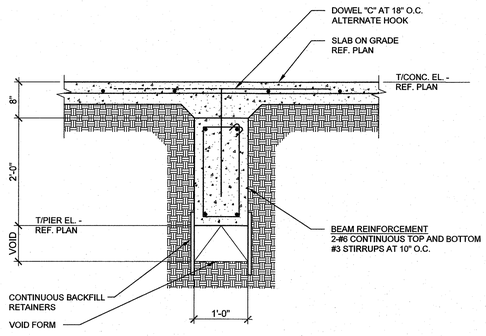

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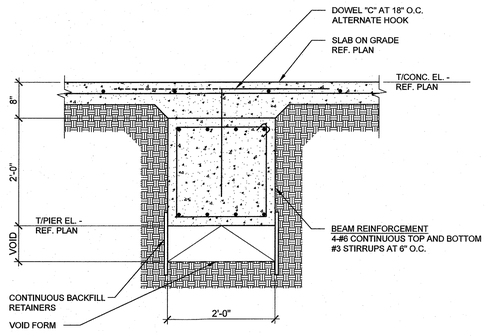

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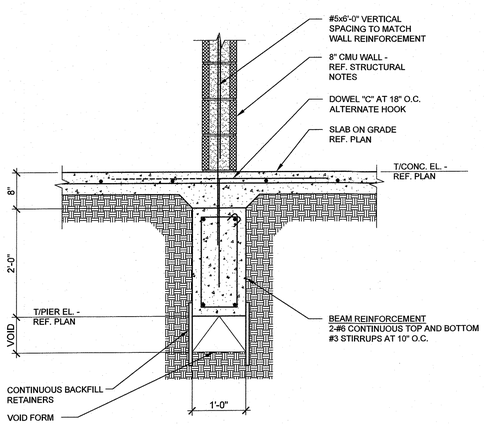

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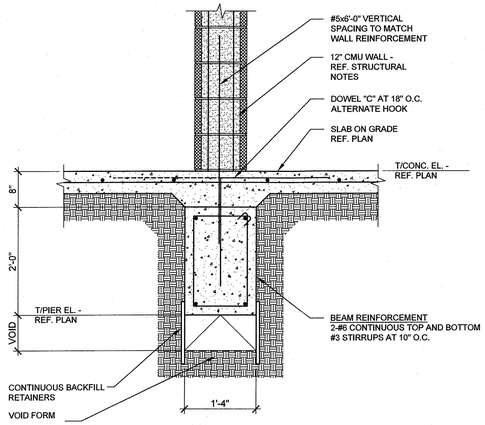

10 **TYPICAL FOUNDATION DETAIL**
NO SCALE

REGISTERED ARCHITECT CLINTON W. FARRIS 28983 STATE OF TEXAS
CLINTON W. FARRIS #28983
DATE SIGNED: 7.28.22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET
GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

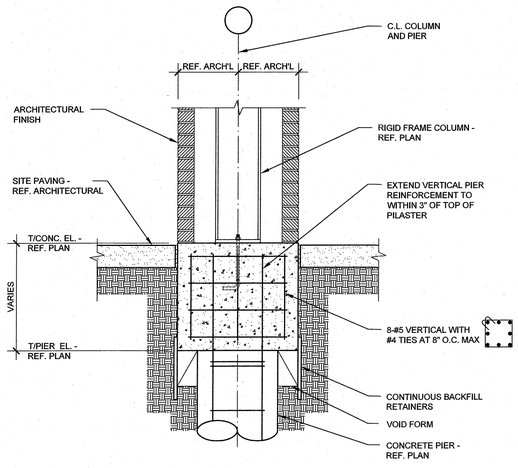
NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S302

STATE OF TEXAS R. TRENT PERKINS 84264 REGISTERED PROFESSIONAL ENGINEER
7/28/2022
THE SEAL APPEARING ON THIS DOCUMENT WAS AUTHORIZED BY R. TRENT PERKINS, P.E. 84264

STRUCTURAL, PLLC
Texas Engineering Firm F-16159
P.O. Box 1599
Rockwall, Texas 75087
Phone 214-293-2503
RTP#: 22062



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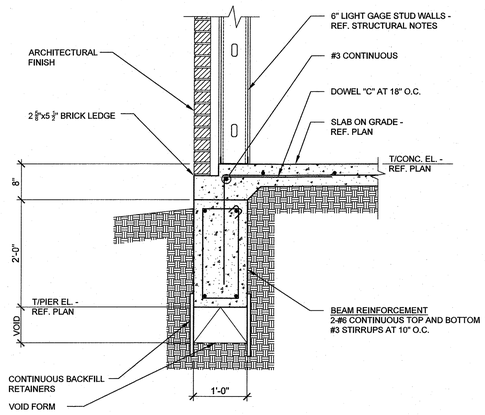

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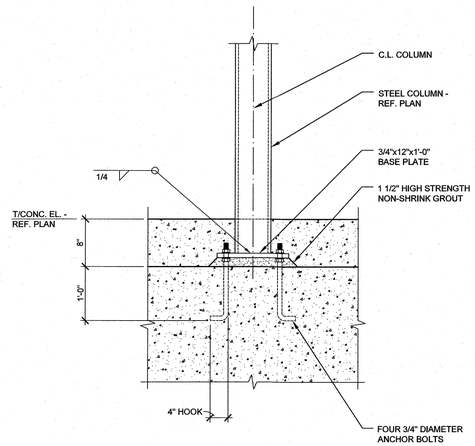

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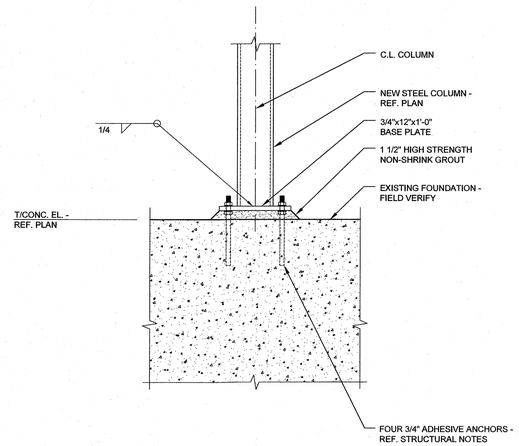

04 **TYPICAL FOUNDATION DETAIL**
NO SCALE

REGISTERED ARCHITECT
CLINTON W. FARRIS
28983
STATE OF TEXAS

CLINTON W. FARRIS #28983
DATE SIGNED: 7.28.22

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ADDITIONS AND RENOVATIONS TO THE

GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.

112 RUSK STREET

GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE
1	CONTRACTOR VE	6/8/21

PROJECT NO.
20864.00

SHEET NO.
S303

STATE OF TEXAS
R. TRENT PERKINS
84264
REGISTERED PROFESSIONAL ENGINEER
7/28/2022

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STRUCTURAL, PLLC
Texas Engineering Firm F-16159
P.O. Box 1599
Rockwall, Texas 75087
Phone 214-293-2503
RTP#: 22062

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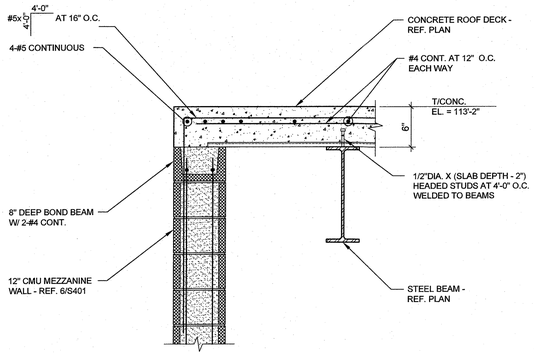
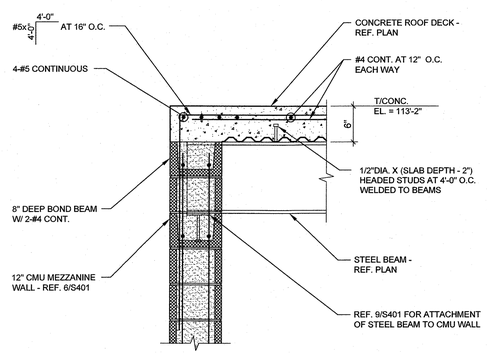


01 **SECTION AT MEZZANINE**
NO SCALE



02 **SECTION AT MEZZANINE**
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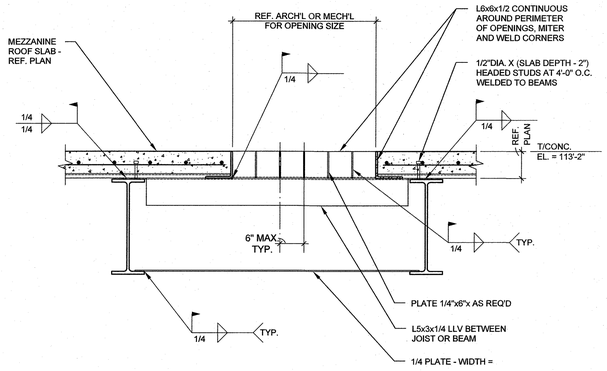

TYPICAL MEZZANINE ROOF OPENING DETAIL
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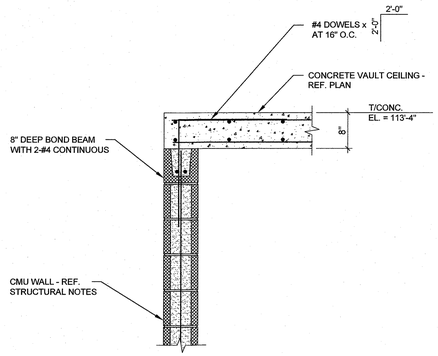

04 **SECTION AT VAULT CEILING**
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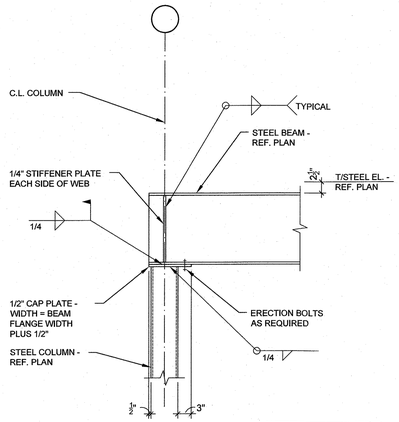
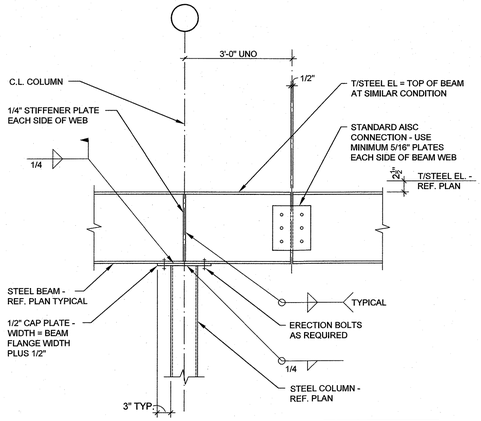


NOTE: JOISTS AND DECK NOT SHOWN FOR CLARITY

05 **TYPICAL BEAM TO COLUMN CONNECTION DETAIL**
NO SCALE



NOTE: JOISTS AND DECK NOT SHOWN FOR CLARITY

06 **TYPICAL BEAM SPLICE DETAIL**
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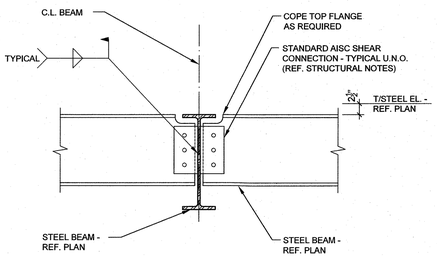
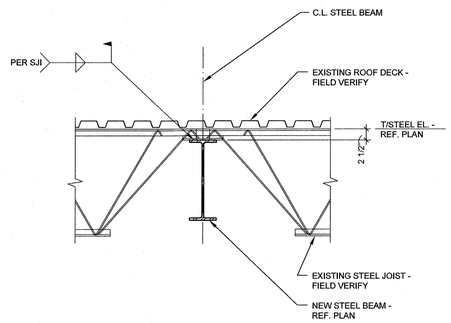
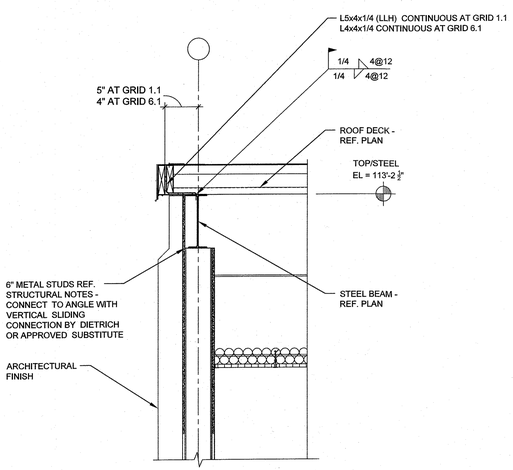


07 **TYPICAL BEAM TO BEAM SHEAR CONNECTION DETAIL**
NO SCALE



08 **TYPICAL ROOF DETAIL**
NO SCALE



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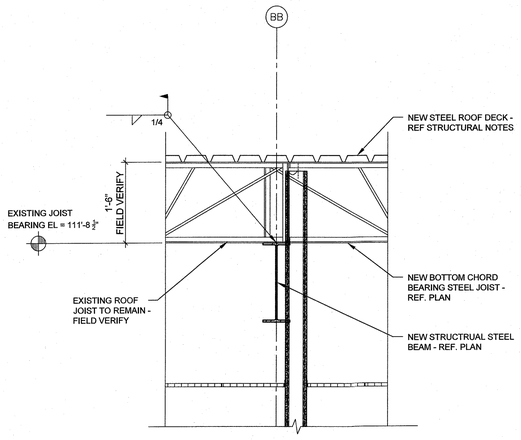

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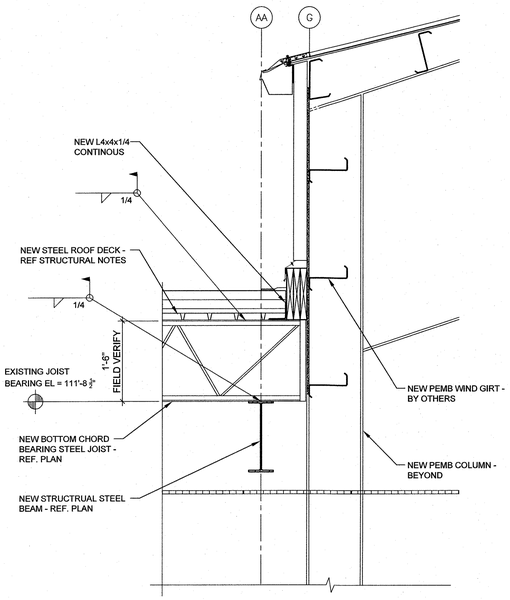

11 **TYPICAL ROOF DETAIL**
NO SCALE

REGISTERED ARCHITECT
CLINTON W. FARRIS
28983
STATE OF TEXAS
CLINTON W. FARRIS #28983
DATE SIGNED: 7.28.22

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ESTABLISHED 1962
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WICHITA FALLS, TEXAS 76302-3599
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ADDITIONS AND RENOVATIONS TO THE
GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.
112 RUSK STREET
GORDON, TEXAS 76453

DRAWN BY: RTP

DATE: 28 JULY 2022

REVISIONS

NO	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S401

STATE OF TEXAS
R. TRENT PERKINS
94264
REGISTERED PROFESSIONAL ENGINEER
7/28/2022

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STRUCTURAL, PLLC
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P.O. Box 1599
Rockwall, Texas 75087
Phone 214-293-2503
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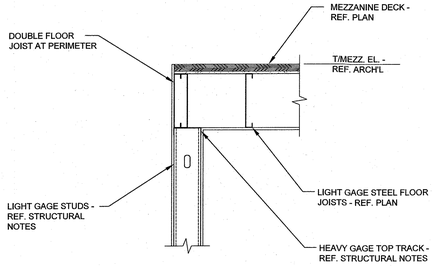

01 TYPICAL WALL DETAIL (JOISTS PARALLEL TO WALL)
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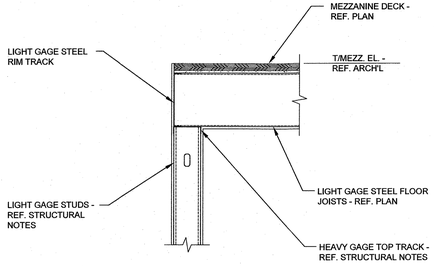

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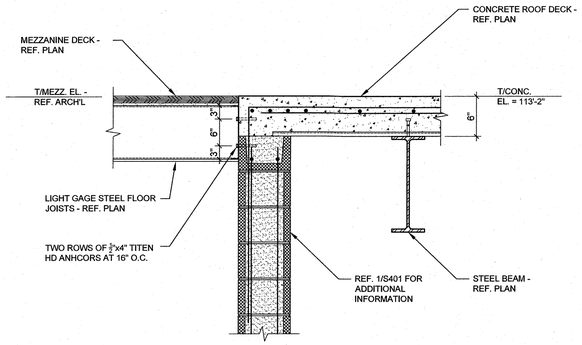
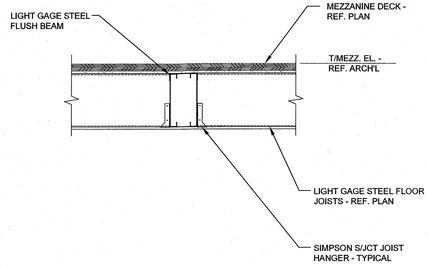
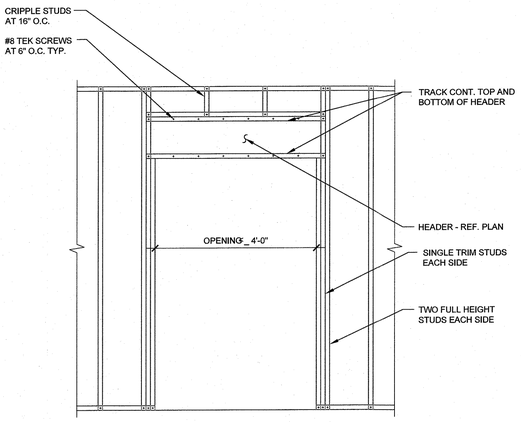
NO SCALE



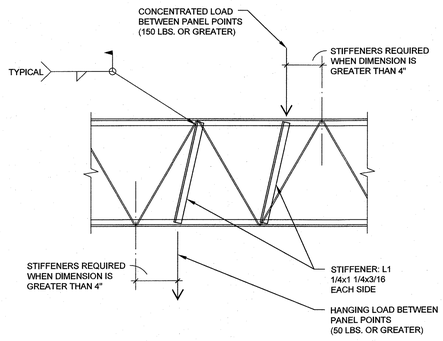
03 SECTION AT MEZZANINE
NO SCALE



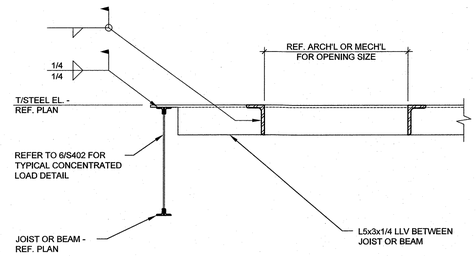
04 TYPICAL JOIST TO FLUSH BEAM CONNECTION
NO SCALE



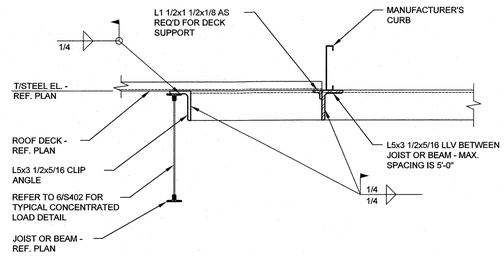
05 TYPICAL OPENING FRAMING AT LIGHTGAGE STEEL STUD WALL - OPENING 4'-0"
NO SCALE



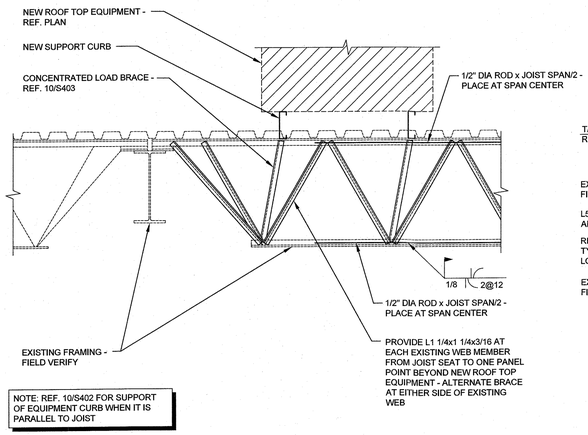
06 TYPICAL JOIST AT CONCENTRATED LOAD DETAIL
NO SCALE



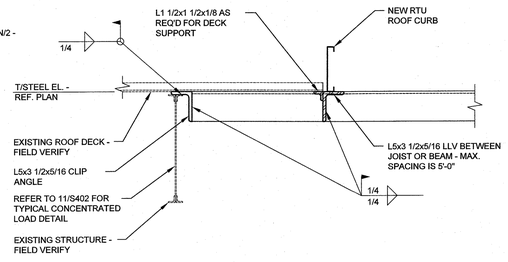
07 TYPICAL ROOF OPENING DETAIL
NO SCALE



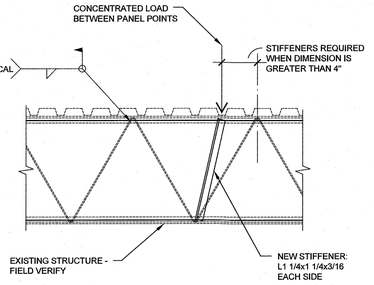
08 TYPICAL HVAC UNIT FRAMING DETAIL
NO SCALE



09 TYPICAL DETAIL TO REINFORCE EXISTING JOISTS FOR ADDITIONAL RTU LOADINGS
NO SCALE



10 TYPICAL HVAC UNIT FRAMING DETAIL
NO SCALE



11 TYPICAL JOIST AT CONCENTRATED LOAD DETAIL
NO SCALE

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ADDITIONS AND RENOVATIONS TO THE
GORDON I.S.D. CAMPUS
FOR GORDON I.S.D.
112 RUSK STREET
GORDON, TEXAS 76453

DRAWN BY: RTP
DATE: 28 JULY 2022

REVISIONS

NO.	DESCRIPTION	DATE

PROJECT NO.
20864.00

SHEET NO.
S402

R. TRENT PERKINS 84264 REGISTERED PROFESSIONAL ENGINEER STATE OF TEXAS
7/28/2022
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