	Required Inspection Verification, or Test	Verification Monitoring Frequency	Type and/or Frequency of Testing	IBC Section & Reference Criteria	Inspector Qualifications
SO A.	ILS Site Preparation			IBC 1705.6	
	1. Visual Observation	Periodic	At the Contractor's expense, instrument readings shall be taken by a licensed surveyor to verify final subgrade elevations and slopes.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740 Licensed Surveyor
	2. Proofrolling Observations	Continuous	Proofrolling shall be monitored by a Geotechnical Engineer. The Geotechnical Engineer shall approve the type of proofrolling equipment and procedures.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
		Periodic	Verify materials below shallow foundations are adequate to achieve the design bearing capacity.	_	
		Periodic	Verify excavations are extended to proper depth and have reached proper material.		
		Periodic	Prior to placement of compacted fill, observe subgrade and verify that site has been properly prepared.		
	<ol> <li>Moisture Conditioning &amp; Recompaction</li> </ol>	Continuous or Periodic	Provide (1) one density test for each 2000 sq. ft. Refer to Notes on Building Pad for Testing Specifications.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
B.	Chemical Injection	Continuous	Quality controlled testing and evaluation prior and subsequent to injection shall be performed by the Geotechnical Engineer to determine effectiveness of the chemical injection process. The Geotechnical Engineer or his representative shall monitor the injection process to verify area coverage, injection depth and review & monitor swell test results.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
C.	During Fill Placement	Continuous or Periodic	Visual Observation: During placement and compaction of fill, Special Inspector shall determine that material being used and the maximum lift thickness comply with project requirements. Perform classification and testing of compacted fill materials. Verify use of proper materials, densities and lift thicknesses during placement and compaction of compacted fill. Pit run materials shall be visually monitored by the lab with additional samples tested each day, or more often if material appears to vary.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
D.	Evaluation of In-place Density of Fill	Continuous or Periodic	Provide (1) one density test for each 2000 sq. ft. Refer to Notes on Building Pad for Testing Specifications.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
E.	Trench Backfilling	Continuous or Periodic	Trench Backfilling: Trench backfilling with clay cap and placing of clay plug shall be monitored by Geotechnical Engineer with a written report sent to Structural Engineer.	Geotechnical Report; Structural Notes	Qualifications based on ASTM D3740
	Required Inspection Verification, or Test	Verification Monitoring Frequency	Type and/or Frequency of Testing	IBC Section & Reference Criteria	Inspector Qualifications
CC		Deviedia		IBC 1705.3	0.1.1.1.5.
A.	Reinfording Steel	Periodic	Provide periodic inspection of reinforcing sizes, spacing, grade of rebar, and placement at the following frequency: Columns: 10% Beams: 30% Joist: 10% Other members: randomly @ 20	IBC 1910.4 ACI 318: 3.5, 7.1-7.7 Structural Notes	Geotechnical Enginee Qualifications based on ASTM E329 & ASTM C1077
В.	Reinforcing Steel Welding		No field welding permitted.	AWS D1.4, ACI 318: 3.5.2 IBC Table 1705.2.2, Item 2b	CWI or Associate CW
C.	Inspection of anchors cast in concrete where allowable loads have been increased or where strength design is used	Periodic	(Increase of allowable loads not recommended)	IBC 1908.5, 1909.1 ACI 318: 8.1.3, 21.2.8	Technician trained in field of work and has at least one year of experience.
D.	Inspection of anchors post-installed in hardened concrete members	Periodic		IBC 1909.1 ACI 318 3.8.6, 8.1.3, 21.2.8	Qualifications based on ASTM C1077
E.	Verify use of required mix design	Periodic	Each concrete placement.	IBC 1904.2, 1910.2, 1910.3 ACI 318 Ch.4, 5.2-5.4	Qualifications based on ASTM C1077
F.	Sampling of fresh concrete.	Continuous Each Concrete Placement	<ol> <li>All concrete testing is to be made after water, if any, is added at site.</li> <li>Provide a set of (4) four cylinders to be taken for every 75 cubic yards of concrete, or fraction thereof, by testing lab.</li> <li>Monitor slump, temperature and air content of concrete and notify delivery driver if slump deviates more than plus or minus 1 inch from recommended value. Contact supplier for further directions.</li> </ol>	IBC 1910.10 ASTM C 172 ASTM C31 ACI 318 5.6, 5.8	Qualifications based on ASTM C1077
G.	Inspection of concrete and shotcrete placement for proper application techniques.	Continuous		IBC 1910.6, 1910.7, 1910.8 ACI 318 Ch. 5.9, 5.10	Qualifications based on ASTM C1077
H.	Inspection for maintenance of specified curing temp & techniques	Periodic	Each concrete placement.	IBC 1910.9 ACI 318 5.11-5.13	Qualifications based on ASTM C1077
I.	Pre-stressed concrete	Continuous	<ol> <li>Application of prestressing forces.</li> <li>Grouting of bonded prestressing tendons in seismic-force resisting systems</li> </ol>	ACI 318: 18.20 ACI 318: 18.18.4	Qualifications based on ASTM C1077
J.	Erection of precast concrete members	Periodic		ACI 318: Ch.16	Technician trained in field of work and has at least one year of experience.
К.	Post-Tensioned Concrete	Periodic	Verify in-situ concrete strength prior to stressing of tendons in post-tensioned concrete and prior to removal of shores and forms from beams and structural slabs.	ACI 318: 6.2	Qualifications based on ASTM E329
L.	Formwork	Periodic	Inspect formwork for shape, location and dimensions of the concrete member being formed.	ACI 318: 6.1.1	Qualifications based on ASTM E329

STRUCTURAL ABBREVIATIONS	ABOVE FINISHED FLOOR AFF ADDITIONAL	CAST-IN-PLACE CEILING CENTERLINE Connx Design Specialty Engineer CLEAR COLUMN CONCRETE CONCRETE MASONRY UNIT CONCRETE MASONRY UNIT CONCRETE MASONRY UNIT CONCRETE MASONRY UNIT CONCRETE MASONRY UNIT DONCRETE MASONRY UNIT DETAIL DETAIL DEAD LOAD DEFORMED BAR ANCHOR DIAGONAL DIAMETER DIMENSION (S)
õ	BUILDING LINEB.L	

	Required Inspection Verification, or Test	Verification Monitoring Frequency		Type and/or Frequency of Testing	IBC Section & Reference Criteria	Inspector Qualifications
PIE	R FOUNDATIONS				IBC 1705.8	
A. 1. 2.	Pier Installation The Geotechnical Engineer shall be present during the excavation of the first pier shaft. All piers monitored by a representative of the Geotechnical Engineer.	Continuous	1. 2. 3. 4.	Observe drilling operations and maintain complete and accurate records for each pier. Address unforeseen subsurface conditions, if any. Verify placement locations and plumbness, confirm element diameters, bell diameters (if applicable), lengths, embedment into bedrock (if applicable) and adequate end-bearing strata capacity. Verify the bearing stratum is encountered at the anticipated depth. Report concrete volumes. Verify conformance with the foundation recommendations provided in the project "Geotechnical Engineering Report" and the Structural Drawings issued for the project. Check reinforcing size, quantity and clearances.	Geotechnical Report; Structural Notes	Geotechnical Engineer Qualifications based on ASTM E329 & ASTM C1077
В.	Additional Inspections	Continuous	1.	For concrete piers, perform additional inspections in accordance with IBC 1705.3		Qualifications based on ASTM E329 & ASTM C1077

technical Engineer fications based STM E329 & M C1077

or Associate CWI

fications based STM E329

GENERAL: The structure has been des partially completed structure and any other supporting ele are made, the contractor mu

DESIGN CRITERIA:

DESIGN CODE:

DESIGN LOADS

Roof Live:

accordance with the contract documents.

See architectural drawings for the following: Size & location of all doors and windows Size & location of all roof openings Floor and Roof finishes

 Details of veneer attachment Location & extent of insulation Electrical conduit runs, boxes, outlets in walls and floors

 Underground concrete ducts, trenches, pits, or manholes Concrete and asphalt site paving.

up of materials.

SHOP DRAWINGS: "exceptions as noted" only.

must bear the Review Stamp of the General Contractor.

necessarv.

The use of reproductions or electronic files of the structural drawings for the preparation of shop drawings is not acceptable without prior written authorization of the EOR. Omission from the shop drawings of any requirements of the contract documents shall not relieve the contractor of the responsibility of complying with the omitted requirements, even if the shop drawings have been reviewed and returned.

FOUNDATION NOTES:

After select fill has been brought to the bottom of the floor slab, line the entire building pad with a 10 mill polyethylene sheet.

**DRILLED STRAIGHT SHAFT FOUNDATIONS:** 

Allowable end bearing: 25,000 psf

on centerline of wall or beam. Elevation of top of pier is noted on drawings. Reference plans and pier schedule for pier size, reinforcing, and penetration into bearing stratum.

and cut as required. Provide minimum 64 bar diameter laps in all vertical pier reinforcing.

material has been reached in accordance with the recommendations given in the geotechnical report. Cast concrete for all drilled shafts within 8 hours of drilling completion.

bearing stratum specified on plan shall be below the temporary casing.

... C.I.P. DRAWING (S) DWG (S) FIELD VERIFY INFORMATION .... MECHANICAL. POUNDS PER SQUARE FOOT ... SCHEDULE (D). SCHED TOP OF BEAM .... .. TOP/BM ..INFO MECH ... F.V. PSF CLG CLG CL CLSE CLR CLR CLR COL CONC CONC CONNX DOUBLE .... FINISHED (ED) INSIDE DIAMETER. MEZZANINE. POUNDS PER SQUARE INCH.. TOP OF FOOTING ... .. TOP/FTG ... FIN . MEZZ SECTION ..... SECT PSI DBL DOWEL (S).... . FIN FL MIDDLE.... POWDER ACTUATED FASTENER.. PAF SHEET .... TOP OF PIER ..... TOP/PIER DWL (S) FINISHED FLOOR ... INTERIOR ..... MID MISCELLANEOUS..... SIMILAR..... TOP OF PIER CAP.... ...TOP/PER CAP FLANGE .... ....FLG .. MISC PRECAST CONCRETE.... P/C SIM JOINT ... PRE ENGINEERED METAL BUILDING..... EACH ..... FLOOR ..... PEMB SPECIFICATION (S).. SPEC (S) TOP OF STEEL .... .. TOS EDGE OF DECK.. FLOOR DRAIN ... JOIST (S)..... SQUARE FOOT (FEET)... EOD .. JST (S) NOMINAL. NOM PREFABRICATED.... PREFAB TOP OF WALL .... .. TOW NOT TO SCALE ..... ELECTRICAL ... FOOTING . STANDARD ... TYPICAL..... TYP ELEC ..FTG NTS STD KIP (1000 LBS) .. PRELIM ELEVATION ... ELEV NUMBER... No. OR # PRELIMINARY ... STEEL ... EMBED GAGE OR GAUGE. KIP PER LINEAR FOOT ... PROJECTION ..... UNLESS NOTED OTHERWISE......UNO EMBEDMENT.. PROJ STIFFENER .. STIFF ... KLF .....CONT .....CJ GALVANIZE (D) .... KIP PER SQUARE FOOT ..... .... KSF ON CENTER.. STIRRUPS .. ENGINEER .... ENGR ...GALV . STIR LIGHTWEIGHT CONCRETE.....LWT CONC VERTICAL..... .. VERT GLUE LAMINATED WOOD BEAM ..... OPPOSITE .... RADIUS .... STRUCTURE .. EQUAL... . GLB STRUCT EQ OPP OPPOSITE HAND ..... EQUIPMENT .. LIVE LOAD ..... REFERENCE (REFER TO)... STRUCTURAL... STRUCT'L GRADE .... REF EQUIP OH .... DET EXPANSION .... EXP GRADE BEAM .... LONGITUDINAL ... OUTSIDE DIAMETER...... O.D. REINFORCE (ING) (ED) (MENT).... REINF SYMMETRICAL .... WELDED WIRE REINFORCING .... ....WWF ...GR BM SYM .... LONG EXPANSION JOINT .... SUBCONTRACTOR ... .... DL LONG LEG HORIZONTAL .... REQUIRED ... REQ'D SUBCONTR WIND LOAD .... E.I .....LLH .....DBA EXISTING ... EXIST HEADED STUD ANCHOR .... LONG LEG VERTICAL ... PERPENDICULAR ... . PERP ROOF DRAIN ... WITH..... . HSA .... LLV RD ..... DIAG ..... DIA EXTERIOR .... EXT HEIGHT .... PLATE..... ROOF OPENING ... R.O. TEMPERATURE .... TEMP WORK POINT ... EXTRA STRONG ..... HOLLOW STRUCTURAL SECTION...... MANUFACTURER ... PLYWOOD.... ...... X-STR ....MFR . PLYWD ROOM..... THICK..... THK WOOD..... .... HSS RM WD .....DIM (S) HORIZONTAL .... MAXIMUM ..... POINT .... TONGUE AND GROOVE.... T & G WIDE FLANGE . .HORIZ ....MAX ROUND.... RND HOOK ..... TOP OF BEAM ..... TOP/BM SAWN JOINT ....

# **GENERAL NOTES**

• The structural design is in accordance with the 2021 International Building Code.

### 20 psf (reducible) Collateral Dead: 15 psf

#### Unless noted otherwise, all concrete shall be normal weight, with a maximum aggregate size of 1", a maximum slump of 5" (6" +/- 1" for piers), and the following 28 day compressive strength:

CONCRETE:

Drilled Straight Shafts

gned to resist design loads only as a completed structure. Applications of construction loads to the shall be considered by the contractor and so included in the design of shoring, bracing, formwork, ments provided for construction of the structure. During erection and until all permanent connections
st provide temporary bracing to brace the structure in all directions.

The engineer shall not have control or charge of, and shall not be responsible for, construction means, methods techniques, sequences, or procedures for safety precautions and programs in connection with the work, for the acts or omission of the contractor, subcontractor, or any other persons performing any of the work, or for the failure of any of them to carry out the work in

The Contractor shall verify the presence, size, and correctness of all openings, slab depressions, recess pockets, and embedded items required prior to starting the work. Notify Architect/Engineer of any potential conflicts with mechanical, electrical, or architectural items and structural elements. No openings shall be permitted through structural elements unless shown on the drawings or reviewed and approved by the Engineer.

Where a section is cut on the drawings, it shall apply to all like or similar conditions (U.N.O).

See mechanical, plumbing, electrical and civil drawings for the following: Pipe runs, sleeves, hangers, trenches, wall and floor openings, etc

Concrete inserts for electrical, mechanical, or plumbing fixtures

Any inspections, special or otherwise, that are required by the building codes, local Building Departments, or these plans, shall be done by a qualified independent inspection company. Job site visits by the Engineer do not constitute, or substitute, inspections. The drawings in the structural documents are not to be scaled for any purpose, including the determination of quantities and the fit

All work is to be done using material from approved shop drawings. The contractor shall submit an electronic PDF copy for review by the Engineer of Record (EOR). Work may begin using materials from shop drawings marked "no exceptions taken" or

The General Contractor shall pre-check all shop drawings before submission to the Engineer for review. All submittal materials

processing and review by the Design Team. This shall include a minimum of ten (10) working days, excluding delivery time, for Engineer's processing and review of shop drawings. Include time for Contractor's resubmission and subsequent review if

structural drawings. Corrections or comments made (or not made) on the shop drawings during this review do not relieve the contractor from compliance with the requirements of the plans, general notes, or specifications. Approval of a specified item shall Design of the metal building superstructure is to be provided by the Metal Building Supplier. The design is to meet the load not include approval of an assembly of which the item is a component.

Signed and sealed calculations for a particular system submitted as part of a shop drawing are reviewed for load criteria and general conformance with the contract documents. Calculation review and comments do not infer a detailed check of the calculations, nor do they relieve the system engineer or the contractor of responsibility.

Foundation Design basis: Geotechnical Report No. 398-24-24 prepared by CMJ Engineering, Inc. dated June 21, 2024. The subgrade shall be prepared in accordance with the geotechnical report recommendations provided in Section 5.3 which calls for the installation of 2 feet of non-expansive select fill over a minimum of 12 feet of moisture conditioned clays.

The subgrade shall be tested, inspected and approved by the Geotechnical Engineer in writing prior to placing concrete. The subgrade preparation shall improve subgrade performance to limit the PVR to 1".

Positive drainage must be provided away from the structure to prevent ponding of water in the select fill, during and following construction. Backfill the exterior face of all grade beams with compacted onsite clays at a minimum of 2 percentage points above optimum moisture and at a minimum of 93 percent of ASTM D 698.

Bearing material: Gray Limestone or Shale located approximately 18 feet below existing grade.

Allowable side shear: 4,000 psf for penetration below a minimum of 3 feet

Piers not specifically located on the plan shall be located on centerline of the column above. Where no column occurs, locate pier

Provide dowels from piers into concrete above per the typical pier detail or sections.

The contractor shall verify depths of piers before pier steel is cut. Pier steel may be delivered to the jobsite in standard lengths

All piers shall be inspected by a representative of a qualified geotechnical laboratory in order to ensure that the proper bearing

If sufficient quantities of water seepage are encountered the use of temporary casing may be necessary for proper installation of shafts. The casing shall have an inside diameter equal to the pier diameter shown in the pier schedule. The penetration into the

Do not allow tops of piers to "mushroom". The top of pier shall be of the specified diameter.

Slabs on Grade 3500 psi Grade Beams 3500 psi 3500 psi Pier Caps

Provide normal weight concrete with cured density of 145 ± 5 pcf, and aggregate conforming to ASTM C33, UNO. All concrete work shall conform to the requirements of the latest edition of ACI 301 Specification for Structural Concrete and ACI 318

3000 psi

Building Code Requirements for Structural Concrete unless noted otherwise in the Contract Documents.

Concrete mix designs shall be prepared by a qualified testing agency or material supplier in accordance with ACI 318. Contractor to submit mix designs for each concrete strength and intended use for review by the Engineer prior to placement. Air entrained admixture is not allowed for concrete to receive a steel trowel finish.

Unless indicated on drawings, concrete beam or wall sections are not designed for construction imposed lateral earth pressures. Provide temporary bracing as required to prevent damage from lateral pressures from earth fill operations or equipment surcharge loads.

Placement of sleeves or openings through beams is not permitted unless indicated on the structural drawings or approved in writing from the Architect

**REINFORCING STEEL:** 

Reinforcing steel shall be new deformed billet steel conforming to ASTM A615, Grade 60.

Detailing of reinforcing steel and accessories shall conform to ACI 315. Required minimum reinforcing cover:

Concrete cast against soil

Grade Beams on void carton 1 1/2" top, 2" sides, 3" bottom Drilled Shafts

1 1/2" top Slabs on Grade

Lap all reinforcing bar splices 30 bar diameters or 2'-0" whichever is greater unless noted otherwise.

Reinforcing call-out legend: #4(2-0,2-0)@12

(2-0.2-0 Length of bar from bend to bend Spacing of bars

or length of bar from bend to end

Provide #4 (2-0,2-0) corner bars to match grade beam exterior reinforcing longitudinal bars.

Hook top and bottom beam reinforcing bars at discontinuous ends.

CURTAIN WALL METAL STUD FRAMING:

The design of the light gauge metal stud curtain wall framing is not included in the structural scope. The general contactor is required to provide shop drawings documenting the curtain wall stud sizes, layout, headers etc. designed in accordance with the architectural performance specifications. Calculations should be included in the shop drawing submittal that are signed and sealed by a professional engineer registered in the State of Texas. The shop drawings should be submitted for record only. The EOR will The General Contractor shall schedule submittals sufficiently in advance of the date required to allow reasonable time for delivery, not stamp nor approve the submittal. We will provide a cursory review for coordination with the project design parameters and will provide comments if necessary. Maximum allowable deflection shall not exceed L/360.

PREFABRICATED METAL BUILDING

The review of the shop drawings by the EOR is only for general compliance with the structural drawings and specifications. This Foundation design and layout has been based on column reactions generated from a preliminary roof frame analysis. Submit final review does not guarantee in any way that the shop drawings are correct or complete, nor does it infer that they supersede the column reactions from the Metal Building Supplier for review of the foundation design prior to the start of foundation construction.

> specifications shown on these drawings as well as all local Building Code requirements. Provide design calculations sealed by an engineer registered in the State of Texas.

1. Framing Type: One-way Rigid frames with pinned column bases.

2. Design and provide framing for suspended loads and collateral dead load as indicated.

3. Calculated values for lateral drift shall not exceed the building eave height divided by 240 (H/240) for Building Code specified wind loading.

4. Maximum allowable deflection for roof purlins and frames under roof live

loads shall not exceed L/240.

5. Maximum allowable horizontal deflection for spandrel beams supporting steel studs with brick veneer and drywall finishes shall not exceed L/360.

6. The Metal Building is to provide a complete system of lateral bracing utilizing rigid frames and horizontal and vertical bracing. Locate vertically braced frames as indicated. It is the Contractor's responsibility to provide temporary bracing as required to insure safe erection of the metal building.

7. All moment resisting steel frames shall be designed with "pinned" bases, transferring no moment to the foundations





2 FOUNDATION PLAN 1/8" = 1'-0"



- FOUNDATION PLAN NOTES

- CONCRETE SLAB SHALL BE 5" THICK, REINFORCED WITH #3@16 EACH WAY.
   REFERENCE TYPICAL DETAIL FOR ADDITIONAL SLAB REINFORCING REQUIRED AT ALL CORNERS
   REFERENCE PIER SCHEDULE FOR SIZE & REINFORCING
   CENTER LINE (C.L.) OF PIER IS ON C.L. OF COLUMN OR GRADE BEAM WHEN THERE IS NO COLUMN. (YPICAL UNITES NOTED OTHERWISE) (TYPICAL UNLESS NOTED OTHERWISE)

LEGEND:

- SJ INDICATES SLAB SAWN JOINT (REFERENCE TYPICAL DETAIL)
- 100-0 INDICATES TOP OF CONCRETE SLAB ELEVATION RELATIVE TO DATUM 100'-0"





## THE PRE-ENGINEERED METAL BUILDING (PEMB) FRAME LOCATIONS ARE SHOWN SCHEMATICALLY BASED ON AN ASSUMED LAYOUT. SEALED SHOP DRAWINGS FROM THE PEMB SUPPLIER SHOULD BE PROVIDED WITH BASE PLATE INFORMATION AND REACTIONS THAT WILL BE USED FOR A FINAL COORDINATION WITH THE FOUNDATION DESIGN AND LAYOUT. THE GENERAL CONTRACTOR SHOULD NOT BEGIN THE PROCESS OF ORDERING REINFORCING STEEL FABRICATION OR FOUNDATION INSTALLATION UNTIL THE PEMB SHOP DRAWINGS HAVE BEEN REVIEWED AND REVISED FOUNDATION DRAWINGS HAVE BEEN ISSUED AS REQUIRED PER THE FINAL BUILDING DESIGN.

FOUNDATION PLAN NOTES

- CONCRETE SLAB SHALL BE 5" THICK, REINFORCED WITH #3@16 EACH WAY.
   REFERENCE TYPICAL DETAIL FOR ADDITIONAL SLAB REINFORCING REQUIRED AT ALL CORNERS
   REFERENCE PIER SCHEDULE FOR SIZE & REINFORCING
   CENTER LINE (C.L.) OF PIER IS ON C.L. OF COLUMN OR GRADE BEAM WHEN THERE IS NO COLUMN. (TYPICAL UNLESS NOTED OTHERWISE)
- LEGEND:
- SJ INDICATES SLAB SAWN JOINT (REFERENCE TYPICAL DETAIL)
- INDICATES TOP OF CONCRETE SLAB
   ELEVATION RELATIVE TO DATUM 100'-0"

A R C H I T E C T U R E SCOTT MARTSOLF - ARCHITECT				
815 W. Daggett Ave. Fort Worth, Texas 76104 Phone: (817) 820-0005				
HnH ENGINEERING, INC. 105 SPROLES DRIVE FORT WORTH, TX 76126 PHONE: (817) 713-4977 WILL@HnHENG.COM TBPE FIRM No. F-13825				
SAINT TERESA OF CALCUTTA CATHOLIC CHURCH				
PARISH HALL				
13517 ALTA VISTA ROAD Fort worth, tx 76262				
BID SET NOT FOR CONSTRUCTION				
Drawing Title:				
FOUNDATION PLAN - ALTERNATE Project No. Date: 2024.151 08/05/2024				
**PRELIMINARY** NOT FOR CONSTRUCTION Sheet No. \$\$ Sheet No. \$\$ \$2.10				



1 SCHEMATIC ROOF FRAMING PLAN 1/8" = 1'-0"

B \_ \_ \_ \_ \_ \_ \_ \_\_\_\_ \_ \_ \_ \_ \_ \_ \_ \_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_\_\_\_\_ \_ \_ \_\_\_ \_ \_ \_\_\_\_ \_\_\_\_\_











8 TYPICAL CONST JOINT IN BM DETAIL NTS

9 TYPICAL GR BM PENETRATION DETAIL NTS























ANCHO	ANCHOR ROD SCHEDULE		
SIZE	EMBEDMENT LENGTH		
5/8" DIA	1'-0"		
3/4" DIA	1'-4"		
7/8" DIA	1'-6"		
1" DIA	1'-8"		
1 1/8" DIA	1'-10"		
1 1/4" DIA	2'-0"		

NOTE: ALL ANCHOR BOLTS TO BE F1554 GR36 OR A36 THREADED ROD





1 <u>SECTION</u> 1" = 1'-0"

2 SECTION 1" = 1'-0"







11 DETAIL 1" = 1'-0"

(12) SECTION 1" = 1'-0"





13 <u>SECTION</u> 1" = 1'-0"



REF \_ 7 / S3.2

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4 <u>SECTION</u> 1" = 1'-0"









14 <u>SECTION</u> 1" = 1'-0"

